DESIGN CRITERIA:

CODE — 2018 INTERNATIONAL BUILDING CODE

ROOF

DEAD:	25 PSF
SNOW:	25 PSF + DRIFT

MEZZANINE

0.6C FORM DECK + 2" CONCRETE:	30 PSF
FRAMING + MEP:	20 PSF
LIVE:	100 PSF

WIND — V = 114 MPH (ULTIMATE) RISK CAT III, EXPOSURE C, GC_{pi} = ±0.18

SEISMIC

RISK CAT:	III
SEISMIC DESIGN CAT:	B
IMPORTANCE FACTOR:	1.25
SOIL CLASSIFICATION:	D (DEFAULT)
S_S:	0.134
S_1:	0.069
S_{DS}:	0.143
S_{D1}:	0.11
SEISMIC FORCE RESISTING SYSTEM:	INTERMEDIATE REINFORCED MASONRY SHEAR WALLS
R:	3½
C_D:	2¼
Ω_0:	2½
ANALYSIS PROCEDURE:	EQUIVALENT LATERAL FORCE METHOD

GENERAL FOUNDATION NOTES:

1. THE FOUNDATION DESIGN IS BASED ON THE GEOTECHNICAL REPORT DATED 09/05/2024, PREPARED BY K AND S ENGINEERS, INC, INC (REPORT NO. 14069).
2. ALL SOIL SUPPORTED FOOTINGS SHALL BE FOUNDED UPON UNDISTURBED, NATURAL SUBGRADE WITH A MINIMUM ALLOWABLE BEARING CAPACITY OF 3000 PSF AS FIELD VERIFIED AND APPROVED BY THE OWNER'S SOIL TESTING LABORATORY. THE BOTTOM OF THE FOOTING ELEVATIONS AND SOIL BEARING CAPACITIES AS SHOWN ON THE DRAWINGS ARE ESTIMATED FROM THE SOIL BORING DATA. FINAL, EXACT ELEVATIONS AND SOIL BEARING CAPACITIES SHALL BE FIELD DETERMINED AND VERIFIED BY THE OWNER'S SOIL TESTING LABORATORY AND REVIEWED BY THE ARCHITECT / ENGINEER DURING CONSTRUCTION.
3. SOIL SUBGRADE FOR ALL FOOTINGS AND SLABS SHALL BE INSPECTED AND APPROVED BY THE OWNER'S SOIL TESTING LABORATORY IMMEDIATELY PRIOR TO PLACING FOUNDATION CONCRETE OR CONCRETE MUD SLABS.
4. ALL FOOTINGS SUBGRADES AS REQUIRED AND ALL SLAB SUBGRADES INCLUDING PIT SLABS SHALL BE COMPACTED TO 95 PERCENT OF MAXIMUM DENSITY AT OPTIMUM MOISTURE CONTENT BASED ON LABORATORY DESIGNATION ASTM D1557. ALL BACKFILL AROUND AND ABOVE ALL FOUNDATION ELEMENTS, FOOTINGS, CAPS, MATS AND PITS SHALL BE COMPACTED TO 90 PERCENT OF MAXIMUM DENSITY AT OPTIMUM MOISTURE CONTENT BASED ON LABORATORY DESIGNATION ASTM D1557.
5. ALL ORGANIC AND / OR OTHER UNSUITABLE MATERIALS SHALL BE REMOVED FROM SUBGRADE AND BACKFILL AREAS AND BACKFILLED WITH ACCEPTABLE GRANULAR FILL, COMPACTED TO 95 PERCENT OF MAXIMUM DENSITY. FILL SHALL BE PLACED IN LIFTS NOT TO EXCEED 12 INCHES IN LOOSE THICKNESS.
6. DO NOT BACKFILL AGAINST BASEMENT WALLS UNTIL GROUND FLOOR AND LOWER LEVEL SLABS HAVE BEEN PLACED AND THE CONCRETE HAS ATTAINED FULL DESIGN STRENGTH.
7. NO MUD SLABS, FOOTINGS OR SLABS SHALL BE PLACED INTO OR AGAINST SUBGRADE CONTAINING FREE WATER, FROST OR ICE. SHOULD WATER OR FROST ENTER A FOOTING EXCAVATION AFTER SUBGRADE APPROVAL, THE SUBGRADE SHALL BE RE-INSPECTED BY THE OWNER'S SOIL TESTING LABORATORY AFTER REMOVAL OF WATER OR FROST.
8. THE CONTRACTOR SHALL PROVIDE ALL NECESSARY MEASURES TO PREVENT ANY FROST OR ICE FROM PENETRATING ANY FOOTING OR SLAB SUBGRADE BEFORE AND AFTER PLACING OF CONCRETE AND UNTIL SUCH SUBGRADES ARE FULLY PROTECTED BY THE PERMANENT BUILDING STRUCTURE.
9. THE CONCRETE FOR EACH ISOLATED FOOTING SHALL BE PLACED IN ONE (1) CONTINUOUS PLACEMENT.
10. ALL SLAB AND FOOTING MUD SLABS SHALL BE THOROUGHLY CLEANED IMMEDIATELY PRIOR TO THE FOUNDATION CONCRETE PLACEMENT.
11. ALL SLABS-ON-GRADE SHALL BE PLACED OVER A VAPOR BARRIER AND A MINIMUM OF 6 INCH COMPACTED GRANULAR FILL MATERIAL OVER A COMPACTED SOIL SUBGRADE.
12. ALL PERIMETER WALL AND COLUMN FOOTINGS SHALL BEAR A MINIMUM OF 3'-6" BELOW FINISHED GRADES SHOWN ON THE CIVIL DRAWINGS.
13. SEE PLUMBING DRAWINGS FOR UNDER FLOOR DRAINAGE SYSTEM AND SPECIAL GRANULAR FILL MATERIALS.
14. SEE ARCHITECTURAL DRAWINGS FOR ALL WATERPROOFING AND DAMPROOFING DETAILS.
15. SEE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.

STEEL JOIST NOTES:

1. OPEN WEB STEEL JOISTS SHALL CONFORM TO THE STEEL JOIST INSTITUTE "STANDARD SPECIFICATIONS" (SJI), LATEST EDITION. JOIST MANUFACTURER'S CERTIFICATION THAT JOISTS COMPLY WITH THE SJI SHALL BE SUBMITTED TO THE ARCHITECT FOR RECORD.
2. ALL JOISTS SHALL REQUIRE CONTINUOUS BRIDGING MEMBERS FASTENED DIRECTLY TO EACH JOIST. BRIDGING SHALL BE AS SHOWN ON THE DRAWINGS OR NOT LESS THAN THAT REQUIRED BY THE SJI SPECIFICATIONS.
3. ALL ROOF JOISTS SHALL BE DESIGNED TO RESIST AN UPWARD LOAD OF 14 PSF. PROVIDE ADDITIONAL BOTTOM CHORD BRIDGING AS REQUIRED TO COMPLY WITH THE STEEL JOIST INSTITUTE TECHNICAL DIGEST #6, "STRUCTURAL DESIGN OF STEEL JOIST ROOFS TO RESIST UPLIFT LOADS".
4. HANGING LOADS FROM THE JOISTS SHALL BE APPLIED ONLY AT PANEL POINTS AND ONLY WITH ACCEPTABLE JOIST HANGER DEVICES.
5. ALL JOISTS SHALL RECEIVE ONE SHOP COAT OF PAINT AFTER 100 PERCENT VISUAL INSPECTION.
6. THE ENDS OF ALL BRIDGING SHALL BE DIAGONAL AND PROPERLY ANCHORED.
7. TOP CHORD JOIST EXTENSIONS SHALL BE DESIGNED BY THE JOIST MANUFACTURER TO SUPPORT THE UNIFORM LOAD FOR THE SPAN OF THE JOIST UNLESS NOTED OTHERWISE.
8. STABILIZE BOTTOM CHORD OF JOISTS AT COLUMN LOCATIONS PER OSHA REQUIREMENTS. COORDINATE STABILIZER PLATE DETAILS WITH STRUCTURAL STEEL FABRICATOR.
9. JOIST MANUFACTURER SHALL SUBMIT SHOP DRAWINGS TO THE ARCHITECT/ENGINEER FOR REVIEW.

EXCAVATION NOTES:

1. THE PERIMETER OF THE GENERAL EXCAVATION SHALL BE RETAINED BY A SOIL RETENTION SYSTEM. THE DESIGN, INSTALLATION, MAINTENANCE AND REMOVAL SHALL BE THE COMPLETE AND SOLE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL PROVIDE ALL MEASURES AND PRECAUTIONS NECESSARY TO PREVENT DAMAGE AND MINIMIZE SETTLEMENT OF EXISTING OR NEW CONSTRUCTION INSIDE OR OUTSIDE THE PROJECT LIMITS. ANY DAMAGE TO NEW OR EXISTING CONSTRUCTION INSIDE OR OUTSIDE THE PROJECT LIMITS, CAUSED BY THE CONSTRUCTION TECHNIQUES OR MOVEMENTS OF THE SOIL RETENTION SYSTEM, IS THE RESPONSIBILITY OF THE CONTRACTOR.
2. THE CONTRACTOR SHALL COORDINATE ALL ELEMENTS OF THE SOIL RETENTION SYSTEM WITH ALL ELEMENTS OF THE PERMANENT BUILDING. THE CONTRACTOR SHALL PREPARE EXCAVATION/SHORING DRAWINGS AND DETAILS PREPARED AND STAMPED BY AN IL S.E.
3. PRIOR TO EXCAVATION OR INSTALLATION OF ELEMENTS OF THE SOIL RETENTION SYSTEM, THE CONTRACTOR SHALL ESTABLISH SURVEY POINTS AROUND THE PERIMETER OF THE AREA TO BE EXCAVATED AND OTHER POINTS UP TO 200 FEET BEYOND THE PERIMETER. THESE POINTS SHALL BE SURVEYED FOR VERTICAL AND HORIZONTAL MOVEMENT AT FREQUENT INTERVALS DURING ACTUAL EXCAVATION AND CONTINUING DURING EACH SUBSEQUENT PHASE OF WORK AND SUBMITTED TO THE ARCHITECT FOR INFORMATION.
4. ALL EXCAVATION SHALL BE BASED ON ENGINEERED DRAWINGS PREPARED BY THE CONTRACTOR INCLUDING PLANS AND SECTIONS OF EXCAVATION SEQUENCES. THE EXCAVATION SEQUENCES SHALL BE CONTROLLED TO MATCH THE REQUIREMENTS OF THE DESIGN OF THE SOIL RETENTION SYSTEM AND TO PERMIT MONITORING OF WALL AND GROUND MOVEMENTS.
5. THE GENERAL EXCAVATION ACROSS THE SITE SHALL NOT EXTEND DEEPER THAN THE SLAB-ON-GRADE SUBGRADE ELEVATION. THE EXCAVATIONS FOR PILE CAPS, CAISSON CAPS, GRADE BEAMS, SPREAD FOOTINGS, MATS, PITS, ETC. SHALL BE EXCAVATED ON AN INDIVIDUAL, LOCALIZED BASIS DOWN FROM THE SLAB-ON-GRADE SUBGRADE LEVEL. THE LAST 6 INCHES OF EACH EXCAVATION SHALL BE EXCAVATED BY HAND TO A TRIM, LEVEL SURFACE.
6. ALL EXCAVATION BELOW THE SLAB LEVEL REQUIRED FOR PITS SHALL BE RETAINED BY LOCALIZED SOIL RETENTION SYSTEMS AS MAY BE NECESSARY BASED ON A DESIGN USING APPROPRIATE EARTH AND HYDRAULIC PRESSURES AND OTHER CONSTRUCTION LOADINGS.
7. THE CONTRACTOR SHALL PROVIDE POSITIVE PROTECTION (MAT / SHEET COVERINGS) FOR ALL EXCAVATION SLOPES TO PROTECT SLOPES FROM INSTABILITY AND DETERIORATION DUE TO RAIN, WIND OR SNOW / ICE.
8. THE CONTRACTOR SHALL PROVIDE SURFACE DRAINAGE CHANNELS AND SUMPS AND SUMP PUMPS TO PROTECT ALL EXCAVATIONS FROM FLOODING. FLOODING OF ANY EXCAVATION AFTER APPROVAL OF THE SUBGRADE WILL BE CAUSE FOR COMPLETE REMOVAL OF CONCRETE MUD SLABS AND COMPLETE PREPARATION AND APPROVAL OF THE SUBGRADE.
9. AFTER COMPLETION OF THE FULL LENGTH OF THE PERIMETER SOIL RETENTION SYSTEM, THE SITE SHALL BE DEWATERED BEFORE (OR AS) THE EXCAVATION PROCEEDS. THE CONTRACTOR SHALL SUBMIT COMPLETE PLANS AND DETAILS OF THE DEWATERING SYSTEM INCLUDING TRENCHES, SUMPS, DEWATERING WELLS, OBSERVATION WELLS, PUMPING SYSTEM, DISPOSAL LOCATION, SETTLING BASIN, MAINTENANCE OR BACK-UP EQUIPMENT, ETC. AT ALL TIMES, THE DEWATERING SYSTEM SHALL MAINTAIN THE WATER LEVEL A MINIMUM OF 3 FEET BELOW THE DEEPEST FOUNDATION SUBGRADE. THE DEWATERING SYSTEM SHALL BE MAINTAINED UNTIL GROUND FLOOR SLABS, PERIMETER WALLS AND WATERPROOFING ARE INSTALLED AND THE PERMANENT BUILDING DRAINAGE SYSTEM IS FULLY OPERATIONAL.
10. THE OWNER'S SOIL TESTING LABORATORY SHALL REVIEW AND MONITOR THE EXCAVATION, DEWATERING AND SOIL RETENTION SYSTEMS. THE CONTRACTOR SHALL PROVIDE, INSTALL AND SURVEY:
    A. VERTICAL AND HORIZONTAL MOVEMENTS OF THE SOIL RETENTION SYSTEM.
    B. BENCH MARKS ADJACENT TO AND AWAY FROM THE SITE PERIMETER FOR VERTICAL AND HORIZONTAL MOVEMENTS.
    C. OBSERVATION WELLS FOR MONITORING WATER LEVELS BELOW GROUND SURFACE.
11. SEE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.

CONCRETE NOTES:

1. ALL CONCRETE WORK SHALL CONFORM TO THE REQUIREMENTS OF ACI 318 AND ACI 301, LATEST EDITIONS. THESE DOCUMENTS SHALL BE AVAILABLE IN THE FIELD OFFICE.
2. EXCEPT WHERE OTHERWISE INDICATED, CONCRETE TYPES AND MINIMUM 28-DAY COMPRESSIVE STRENGTHS SHALL BE AS FOLLOWS:

SLAB-ON-GRADE	4000 PSI REGULAR WEIGHT
SLABS ON METAL DECK	4000 PSI REGULAR WEIGHT
FOUNDATION WALLS	4000 PSI REGULAR WEIGHT
ALL OTHER	4000 PSI REGULAR WEIGHT

3. CEMENT SHALL CONFORM TO ASTM C150 TYPE 1. USE ONLY ONE BRAND OF CEMENT FOR ALL EXPOSED TO VIEW CONCRETE. AGGREGATES SHALL CONFORM TO ASTM C33 (REGULAR WEIGHT). ALL CONCRETE SHALL CONTAIN AN APPROVED WATER REDUCING ADMIXTURE. ALL EXPOSED CONCRETE SHALL BE AIR-ENTRAINED. NO CALCIUM CHLORIDE SHALL BE USED IN ANY CONCRETE.
4. REINFORCING BARS SHALL CONFORM TO ASTM A615, GRADE 60, ALL WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185.
5. ALL CONCRETE REINFORCEMENT SHALL BE DETAILED, FABRICATED, LABELED, SUPPORTED AND SPACED IN FORMS AND SECURED IN PLACE IN ACCORDANCE WITH THE PROCEDURES AND REQUIREMENTS OUTLINED IN THE LATEST EDITION OF THE "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES", ACI 315. BAR SUPPORTS IN CONTACT WITH EXPOSED SURFACES SHALL BE PLASTIC TIPPED.
6. CHECKED SHOP DRAWINGS SHOWING REINFORCING DETAILS, INCLUDING STEEL SIZES, SPACING AND PLACEMENT SHALL BE SUBMITTED TO THE ARCHITECT FOR REVIEW PRIOR TO FABRICATION.
7. THE CONTRACTOR SHALL SUBMIT DETAILED DRAWINGS SHOWING THE LOCATIONS OF ALL CONSTRUCTION JOINTS, REVEALS, CURBS, SLAB DEPRESSIONS, SLEEVES, OPENINGS, ETC.
8. ALL REINFORCING SPLICES SHALL CONFORM TO THE REQUIREMENTS OF ACI 318, LATEST EDITION, MINIMUM 48 BAR DIAMETER LAP FOR #6 BAR OR LESS AND 60 BAR DIAMETER LAP FOR #7 BAR OR LARGER. ALL WELDED WIRE FABRIC SHALL BE LAPPED TWO (2) FULL MESH PANELS AND TIES SECURELY. WHERE REQUIRED, DOWELS SHALL MATCH SIZE AND NUMBER OF MAIN REINFORCING, UNLESS NOTED OTHERWISE.
9. ALL WALLS AND STRUCTURAL SLABS SHALL BE REINFORCED WITH AT LEAST #4 @ 12" EACH WAY, EACH FACE, UNLESS NOTED OTHERWISE. ALL SLABS-ON-GRADE SHALL BE REINFORCED WITH AT LEAST ONE (1) LAYER OF 6x6 W2.9xW2.9 W.W.F., UNLESS NOTED OTHERWISE. PROVIDE ONE ONE (1) LAYER OF 6x6 W1.4xW1.4 W.W.F. CONTINUOUS IN ALL CONCRETE FILLS OVER THE STRUCTURAL SLAB.
10. ADDITIONAL BARS SHALL BE PROVIDED AROUND ALL FLOOR AND WALL OPENINGS, AS SHOWN ON DETAILS.
11. ALLOW A MINIMUM OF THREE (3) HOURS BETWEEN PLACEMENT OF CONCRETE FOR COLUMNS, WALLS OR PIERS AND PLACEMENT OF CONCRETE ON THE ADJACENT FLOOR.
12. SEE FLOOR PLANS, SCHEDULES AND SPECIFICATIONS FOR CAMBER REQUIREMENTS.
13. UNLESS NOTED OTHERWISE, ALL MECHANICAL, PLUMBING AND ELECTRICAL EQUIPMENT PADS SHALL BE REINFORCED WITH AT LEAST ON (1) LAYER OF 6x6 W4xW4 W.W.F. SEE HVAC, PLUMBING AND ELECTRICAL DRAWINGS FOR ADDITIONAL REINFORCING REQUIREMENTS OF PADS.
14. CONSTRUCTION JOINTS IN ALL FOUNDATION WALLS SHALL BE NOT FURTHER APART THAN 40 FEET IN ANY DIRECTION. ALL CONSTRUCTION JOINTS SHALL BE WIRE BRUSHED, CLEANED AND MOISTENED IMMEDIATELY PRIOR TO PLACING NEW CONCRETE. SEE DRAWINGS FOR CONSTRUCTION JOINT DETAILS.
15. PLACE ALL SLABS-ON-GRADE WITH AN APPROVED PATTERN AND SEQUENCE OF CONSTRUCTION AND CONTROL JOINTS TO MINIMIZE SHRINKAGE CRACKS. THE MAXIMUM SPACING BETWEEN JOINTS SHALL BE 15 FEET. A SUGGESTED ARRANGEMENT AND DETAILS ARE SHOWN ON THE DRAWINGS.
16. CONCRETE TESTING WILL BE PERFORMED BY THE OWNER'S TESTING LABORATORY IN ACCORDANCE WITH ACI 301 LATEST EDITION CHAPTER 16, EXCEPT AS FOLLOWS: FOR COMPRESSIVE STRENGTH TEST, TAKE ONE SET OF FOUR (4) SPECIMENS FOR EACH 50 CUBIC YARDS OR FRACTION THEREOF OF EACH CONCRETE CLASS PLACED IN ANY ONE DAY. TEST ONE (1) SPECIMEN AT 7 DAYS, TWO (2) SPECIMENS AT 28 DAYS, AND KEEP ONE IN RESERVE.
17. ALL CONCRETE WORK SHALL BE IN CONFORMANCE WITH ACI 306 LATEST EDITION AND AS AMENDED BY THE VILLAGE OF NEW LENOX. (1)

STRUCTURAL MASONRY NOTES:

1. ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES" ACI 530/ ASCE 5/ TMS 402 AND "SPECIFICATIONS FOR MASONRY STRUCTURES" ACI 530.1/ASCE 6/TMS 602, LATEST EDITION.
2. A. MASONRY UNITS: HOLLOW LOAD BEARING CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C90, GRADE N, TYPE 1, WITH A MINIMUM ULTIMATE STRENGTH OF 2150 PSI ON THE NET SECTION.
   B. MORTAR: MORTAR SHALL BE PORTLAND CEMENT / LIME TYPE N AND CONFORM TO ASTM C270.
   C. GROUT: GROUT FOR REINFORCED LOAD BEARING MASONRY SHALL CONFORM TO ASTM C476 AND SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 2500 PSI.
   D. REINFORCING BARS: REINFORCING BARS FOR REINFORCED MASONRY SHALL CONFORM TO ASTM A615, GRADE 60.
3. VERTICAL CELLS TO BE FILLED WITH GROUT SHALL BE ALIGNED TO PROVIDE A CONTINUOUS UNOBSTRUCTED OPENING OF THE DIMENSIONS SHOWN ON THE PLANS. CELLS WHICH WILL CONTAIN VERTICAL REINFORCEMENT SHALL HAVE A MINIMUM OF TWO (2) INCH CLEAR OPENING.
4. GROUT FOR FILLING REINFORCED OR NON-REINFORCED CELLS SHALL BE FLUID AND PLACED BY ACCEPTABLE PRESSURE GROUTING PROCEDURES.
5. GROUT FOR FILLING REINFORCED OR NON-REINFORCED CELLS SHALL BE PLACED IN A MAXIMUM FOUR (4) FOOT LIFTS AND CONSOLIDATED IN PLACE BY VIBRATION OR OTHER METHODS WHICH INSURE COMPLETE FILLING OF THE CELLS. ALL CELLS CONTAINING REINFORCING BARS AND / OR STUDS SHALL BE FULLY GROUTED.
6. HOLLOW UNITS SHALL BE LAID WITH FULL MORTAR COVERAGE ON HORIZONTAL AND VERTICAL FACE SHELLS EXCEPT THAT WEBS SHALL ALSO BE BEDDED WHERE THEY ARE ADJACENT TO CELLS TO BE REINFORCED AND / OR FILLED WITH GROUT, IN THE STARTING COURSE ON FOOTINGS AND SOLID FOUNDATION WALLS AND IN NON-REINFORCED OR GROUTED PIERS, PILASTERS AND COLUMNS.
7. SOLID MASONRY UNITS SHALL BE LAID WITH FULL HEAD AND BED JOINTS.
8. POINTS OF BEARING SHALL BE ON TWO (2) COURSES OF SOLID MASONRY OR TWO (2) COURSES OF HOLLOW MASONRY GROUTED SOLID 32" LONG UNLESS NOTED OTHERWISE.
9. ALL CUTTING AND FITTING OF MASONRY, INCLUDING THAT REQUIRED TO ACCOMMODATE THE WORK OF THE OTHER TRADES, SHALL BE DONE WITH MASONRY SAWS.
10. CHASES SHALL BE BUILT INTO NEW WALLS, NOT CUT IN. CHASES SHALL BE PLUMB AND SHALL BE A MINIMUM OF ONE (1) MASONRY UNIT LENGTH FROM JAMBS OF WALL OPENINGS. NO CHASES OTHER THAN THOSE SHOWN ON THE DRAWINGS SHALL BE CONSTRUCTED WITHOUT PRIOR REVIEW OF THE ARCHITECT / ENGINEER.
11. REINFORCED MASONRY:
    A. ALL WALLS AND PIERS SHALL HAVE HORIZONTAL JOINT REINFORCEMENTS AT 16" O.C. CONSISTING OF TWO (2) 9 GAGE RODS WITH 9 GAGE CROSS TIES AT 16" O.C., GALVANIZED WITH 0.8 OZ. ZINC COATING, ASTM A116, CLASS 3 (2 RODS IN C.M.U. AND ONE ROD IN FACE BRICK).
    B. THE MINIMUM CLEAR DISTANCE BETWEEN PARALLEL BARS EXCEPT IN COLUMNS SHALL BE EQUAL TO THE NOMINAL DIAMETER OF THE BAR.
    C. VERTICAL REINFORCEMENT SHALL BE LAP SPLICED A MINIMUM OF 48 BAR DIAMETERS (NOT LESS THAN 2'-0") WHERE REQUIRED.
    D. ALL BARS SHALL BE COMPLETELY EMBEDDED IN MORTAR OR GROUT. ALL BARS SHALL HAVE A COVERAGE OF MASONRY NOT LESS THAN: 2" FOR BARS LARGER THAN #5; 1½" FOR #5 BARS AND SMALLER
12. PROVIDE ADEQUATE, TEMPORARY BRACING AS REQUIRED DURING CONSTRUCTION TO WITHSTAND LATERAL LOADS AND THE PRESSURES OF FLUID GROUT.
13. CONCRETE MASONRY SHALL BE PROTECTED FROM ABSORBING MOISTURE AND WATER WHILE AT THE PLANT, DURING SHIPMENT AND AT THE SITE DURING CONSTRUCTION.
14. ANCHORS, WALL PLUGS, ACCESSORIES AND OTHER ITEMS TO BE BUILT IN SHALL BE INSTALLED AS MASONRY WORK PROCEEDS. PROVIDE CONTINUOUS MASONRY BOND BEAMS REINFORCED WITH 2 - #5 REINFORCING BARS AT EACH FLOOR AND ROOF LEVEL. SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL ITEMS.
15. ALL MASONRY SHALL BE LAID IN RUNNING BOND UNLESS NOTED OTHERWISE.
16. SEE SPECIFICATIONS FOR OTHER REQUIREMENTS.
17. ALL INTERIOR NON-LOAD BEARING WALLS SHALL BE LATERALLY BRACED AT THE TOP OF THE WALL UNLESS NOTED OTHERWISE. THE BRACING SHALL ALLOW FOR VERTICAL DEFLECTION OF THE STRUCTURE ABOVE WITHOUT BEARING ON THE WALL.

METAL DECK NOTES:

1. METAL ROOF DECK SHALL BE GALVANIZED OR PAINTED EXCEPT WHERE GALVANIZING IS SPECIFICALLY REQUIRED. GALVANIZED SHEET STEEL SHALL CONFORM TO ASTM A446 GRADE A AND ASTM A525 WITH A MINIMUM G30 COATING UNLESS OTHERWISE NOTED. SHEET STEEL TO BE PAINTED SHALL CONFORM TO ASTM A611 GRADE C.
2. STEEL FORM DECK ("CENTERING") SHALL BE GALVANIZED USING SHEET STEEL CONFORMING TO ASTM A446 GRADE A, B, C, D OR E AND ASTM A525 WITH A MINIMUM G30 COATING UNLESS OTHERWISE NOTED.
3. METAL DECK SECTION PROPERTIES SHALL BE COMPUTED IN ACCORDANCE WITH AISI "SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS". THE MINIMUM THICKNESS OF ROOF DECK SHALL BE 22 GAGE. THE MINIMUM THICKNESS OF FORM DECK SHALL BE 26 GAGE.
4. THE FABRICATOR / ERECTOR SHALL PROVIDE ENGINEERING CALCULATIONS, PUBLISHED MANUFACTURER'S DATA AND INDEPENDENTLY CERTIFIED LOAD TEST DATA VERIFYING THE SPECIFIED DECK REQUIREMENTS TO THE ARCHITECT FOR REVIEW. PROVIDE ENGINEERED AND CHECKED SHOP DRAWINGS INDICATING LOCATION, GAGE AND SIZE OF EACH PIECE OF DECKING. THE DRAWINGS SHALL CLEARLY SHOW WELDING DETAILS TO STRUCTURAL FRAMING AND SIDE LAP CONNECTION DETAILS.
5. ALL DECKING SHALL BE WELDED TO STRUCTURAL STEEL BY QUALIFIED WELDERS USING PRE-QUALIFIED PROCEDURES. THE ERECTOR SHALL ESTABLISH A WELDING PROCEDURE FOR THE PLUG WELD OF THE STEEL DECKING TO THE STRUCTURAL STEEL FOR THE PARTICULAR GAGE USED. PRIOR TO THE START OF ERECTION OF THE STEEL DECK, EACH WELDER SHALL BE QUALIFIED USING THIS PROCEDURE AS WITNESSED BY THE OWNER'S STRUCTURAL STEEL TESTING LABORATORY. THE OWNER'S TESTING LABORATORY WILL VISUALLY INSPECT THE WELDS IN THE FIELD.
6. FASTENING OF METAL DECK TO THE STRUCTURAL STEEL SHALL BE CAPABLE OF RESISTING DIAPHRAGM FORCES. THE FOLLOWING LIMITS SHALL NOT BE EXCEEDED:
    A.) ALL METAL DECKING SHALL BE WELDED AT 12" (6" AT THE PERIMETER) MAXIMUM ON CENTER TO THE SUPPORTING STEEL WITH ⅝" DIAMETER WELDS.
    B.) NO. 10 TEK SIDE LAP SCREWS SHALL BE FASTENED AT 36 O.C. MAXIMUM ON CENTER (1 SCREW MIN. PER SPAN)
    C.) SEE PLAN FOR ADDITIONAL PLAN FASTENING REQUIREMENTS.
7. PROVIDE, AS REQUIRED, ALL RIDGE AND VALLEY PLATES, CLOSURES, CANT STRIPS, SUMP PLATES AT PIPING PENETRATIONS. PROVIDE SUPPLEMENTAL FRAMING AT OPENINGS AS REQUIRED FOR SUPPORT OF THE METAL DECK. ALL OPENINGS SHALL BE COORDINATED WITH THE ARCHITECTURAL AND MECHANICAL DRAWINGS.
8. NO LOADS EXCEEDING 50 LBS. SHALL BE PERMITTED TO BE HUNG FROM ANY METAL DECKING. ALL HANGERS FOR DUCTWORK, PIPING, ETC. SHALL BE HUNG DIRECTLY FROM STRUCTURAL STEEL WORK OR SUPPLEMENTARY MEMBERS OR ANCHORS EMBEDDED IN THE CONCRETE. ALL HANGING LOAD DETAILS SHALL BE SUBMITTED FOR REVIEW.

STRUCTURAL STEEL NOTES:

1. ALL STRUCTURAL STEEL WORK SHALL CONFORM TO THE AISC "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS", LATEST EDITION, AND THE AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES", LATEST EDITION, EXCEPT AS MODIFIED BELOW OR IN THE SPECIFICATIONS.
2. ALL STRUCTURAL STEEL W SHAPES SHALL CONFORM TO ASTM A572 OR A992 GRADE 50. ALL OTHER STRUCTURAL STEEL SHAPES, PLATES AND BARS SHALL CONFORM TO ASTM A36 GR 36, UNLESS NOTED OTHERWISE. COLD FORMED TUBING SHALL CONFORM TO ASTM A500 GRADE C. PIPES SHALL CONFORM TO ASTM A53 TYPE E OR S. ANCHOR BOLTS SHALL CONFORM TO ASTM F1554 GRADE 36 AND BE COMPATIBLE WITH E70XX ELECTRODES
3. ALL BOLTS (OTHER THAN ANCHOR BOLTS), NUTS AND WASHERS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A325. ALL BOLTS SHALL BE 3/4 INCH DIAMETER, MINIMUM. BOLTS USED IN LATERAL LOAD RESISTING CONNECTIONS SHALL BE SLIP CRITICAL TYPE, DESIGNED FOR INDICATED FORCES WITHOUT STRESS INCREASES.
4. ALL WELDING SHALL BE DONE BY QUALIFIED WELDERS AND SHALL CONFORM TO AWS D1.1 "STRUCTURAL WELDING CODE", LATEST EDITION. ALL WELDING ELECTRODES SHALL BE E70XX.
5. ALL CONNECTIONS SHALL BE DESIGNED AND DETAILED BY THE FABRICATOR. THE CONNECTIONS SHALL BE DESIGNED BY, OR UNDER THE SUPERVISION OF, A LICENSED STRUCTURAL ENGINEER IN THE STATE OF ILLINOIS. DETAILING SHALL BE PERFORMED USING RATIONAL ENGINEERING DESIGN AND STANDARD PRACTICE IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. THE GENERAL DETAILS SHOWN ON THE DRAWINGS ARE CONCEPTUAL ONLY AND DO NOT INDICATE THE REQUIRED NUMBER OF BOLTS OR WELD SIZES, UNLESS SPECIFICALLY NOTED. ADVISE THE ARCHITECT IMMEDIATELY IF THE INFORMATION ON THE DRAWINGS IS NOT SUFFICIENT FOR COMPLETE DESIGN OF CONNECTIONS.
6. THE FABRICATOR / ERECTOR SHALL SUBMIT TO THE ARCHITECT FOR REVIEW, ENGINEERED AND CHECKED DRAWINGS SHOWING SHOP FABRICATION DETAILS, FIELD ASSEMBLY DETAILS AND ERECTION DIAGRAMS FOR ALL STRUCTURAL STEEL. WITH EACH SUBMITTAL OF SHOP DRAWINGS, THE FABRICATOR'S ENGINEER SHALL CERTIFY THAT THE CONNECTIONS HAVE BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AISC SPECIFICATIONS AND THE CONTRACT DOCUMENTS. CERTIFIED MILL TEST REPORTS SHALL ALSO BE SUBMITTED.
7. MINIMUM SHEAR CAPACITIES: CONNECTIONS SHALL BE DESIGNED FOR THE BEAM REACTIONS INDICATED. IN CASES WHERE REACTIONS ARE NOT INDICATED, PROVIDE AT LEAST ONE HALF OF THE UNIFORM LOAD CARRYING CAPACITY OF THE BEAM WITH THE ASSUMPTION OF FULLY BRACED COMPRESSION FLANGE.
8. THE DEPTH OF A SIMPLE SHEAR CONNECTION SHALL NOT BE LESS THAN ONE HALF OF THE NOMINAL DEPTH OF THE BEAM. THE MINIMUM NUMBER OF BOLTS PER CONNECTION SHALL BE TWO (2).
9. CONNECTIONS OF BEAMS FRAMING INTO COLUMNS SHALL BE CAPABLE OF RESISTING AN AXIAL FORCE IN ORDER TO BRACE THE COLUMN. THE BRACING FORCE (IN KIPS) SHALL BE TAKEN AS 0.127 TIMES THE NOMINAL WEIGHT IN POUNDS PER LINEAL FOOT OF THE COLUMN, UNLESS NOTED OTHERWISE. THE BRACING FORCE ACTS IN BOTH PRINCIPLE AXES OF THE COLUMN, AND MAY BE RESISTED BY A COMBINATION OF BEAM CONNECTIONS.
10. SHOP AND FIELD TESTING OF WELDS AND BOLTS SHALL BE AS FOLLOWS:
    A.) ALL WELDS SHALL BE VISUALLY INSPECTED.
    B.) FILLET WELDS: TWENTY-FIVE (25) PERCENT OF THE FILLET WELDS, SELECTED AT RANDOM SHALL BE MEASURED, AND TEN (10) PERCENT SELECTED AT RANDOM SHALL BE CHECKED BY MAGNETIC PARTICLE FOR FINAL PASS ONLY.
    C.) PENETRATION WELDS: ULTRASONICALLY TEST 100 PERCENT OF ALL FULL PENETRATION WELDS, AND ALL PARTIAL PENETRATION COLUMN SPLICE WELDS. IF THE WELDS MADE BY AN INDIVIDUAL WELDER ARE CONSISTENTLY SATISFACTORY, TESTING OF THAT INDIVIDUAL'S WELDS MAY BE REDUCED TO 50 PERCENT.
    D.) BOLTED CONNECTIONS: CHECK BY CALIBRATED TORQUE WRENCH 25 PERCENT OF BOLTS IN EACH CONNECTION, BUT NOT LESS THAN TWO (2) BOLTS PER CONNECTION.
    E.) THE OWNER'S TESTING AGENCY SHALL PERFORM ALL SHOP AND FIELD INSPECTIONS AND TESTING AS OUTLINED ABOVE.
    F.) THE STRUCTURAL STEEL FABRICATOR AND ERECTOR SHALL SCHEDULE ALL WORK TO ALLOW THE ABOVE TESTING REQUIREMENTS TO BE COMPLETED.
11. ALL BEAMS SHALL BE FABRICATED WITH THE NATURAL CAMBER UP. PROVIDE CAMBER, OR SHORING AS INDICATED ON THE DRAWINGS.
12. AFTER FABRICATION, ALL STEEL SHALL BE CLEANED OF ALL RUST, LOOSE MILL SCALE AND OTHER FOREIGN MATERIALS. STRUCTURAL STEEL EXPOSED TO VIEW IS TO BE PAINTED WITH SHOP PRIMER. STEEL EXPOSED TO THE WEATHER SHALL BE BLAST CLEANED (SSPC-SP6) AND GIVEN SHOP PRIME AND FIELD FINISH COATS OF PAINT AS SPECIFIED.
13. THERE SHALL BE NO FIELD CUTTING OF STRUCTURAL STEEL MEMBERS FOR THE WORK OF OTHER TRADES WITHOUT THE PRIOR APPROVAL OF THE ARCHITECT.
14. ALL STRUCTURAL STEEL EMBEDDED IN OR ADJACENT TO MASONRY SHALL HAVE MASONRY ANCHORS AT 16" O.C. VERTICAL AND 24" O.C. HORIZONTAL.

LINTEL NOTES:

1. LINTELS SHALL BE PROVIDED FOR ALL OPENINGS AS INDICATED ON THE DRAWINGS. IN ADDITION, LINTELS ARE REQUIRED FOR MECHANICAL, ELECTRICAL OR PLUMBING OPENING IN A MASONRY WALL WITH A WIDTH GREATER THAN 12".
2. LINTELS SHALL HAVE A MINIMUM BEARING OF 8 INCHES FOR SPANS UP TO 8'-0" AND 16" FOR SPANS GREATER THAN 8'-0" UNLESS NOTED. LINTELS IN NON-LOAD BEARING WALLS SHALL BE OF THE SIZES LISTED BELOW.
3. STEEL LINTELS SHALL BE USED FOR ALL BRICK OPENINGS. CMU OPENINGS IN NON-LOAD BEARING WALLS MAY BE SPANNED WITH EITHER A STEEL LINTEL OR MASONRY LINTEL BLOCK AT THE CONTRACTORS OPTION. MASONRY LINTEL INFORMATION LEFT BLANK INDICATES THAT A STEEL LINTEL IS REQUIRED. FOR OPENINGS 2'-0" OR LESS, PROVIDE ⅜" PLATE x WALL THICKNESS LESS 1", BEAR 8" EACH END.

4" WALL SPAN	STEEL LINTEL	MASONRY LINTEL
2'-0" - 4'-0"	∠ 3½x3½x5/16	
4'-0" - 6'-0"	∠ 5x3½x⅜ (LLV)	
6'-0" - 8'-0"	∠ 6x3½x⅜ (LLV)	
6" WALL SPAN		
2'-0"- 4'-0"	WT 4x8	8" LINTEL BLOCK W/ 1-#4
4'-0" - 6'-0"	WT 5x15	8" LINTEL BLOCK W/ 2-#5
6'-0" - 8'-0"	WT 7x11.0	16" LINTEL BLOCK W/2-#5
8" WALL SPAN		
2'-0" - 4'-0"	2-∠ 3½ x3½x5/16	8" LINTEL BLOCK W/ 2-#4
4'-0" - 6'-0"	2-∠ 5x3½x⅜ (LLV)	16" LINTEL BLOCK W/ 2-#4
6'-0" - 8'-0"	2-∠ 6x3½x⅜ (LLV)	16" LINTEL BLOCK W/2-#5
8'-0" - 12'-0"	W8x18 W/ ⅜" PLATE	
12" WALL SPAN		
2'-0" - 4'-0"	3-∠ 3½x3½x5/16	8" LINTEL BLOCK W/ 2-#4
4'-0" - 6'-0"	3-∠ 5x3½x⅜ (LLV)	16" LINTEL BLOCK W/ 2-#4
6'-0" - 8'-0"	W8x24 W/ ⅜" PLATE	16" LINTEL BLOCK W/2-#5
8'-0" - 12'-0"	W12x26 W/ ⅜" PLATE	
CAVITY WALLS		
UP TO 8'-0"	W8x24 W/ ⅜" PLATE	
8'-0" TO 12'-0"	W12x26 W/ ⅜" PLATE (PLATE WIDTH IS 1" LESS THAN ACTUAL MASONRY WIDTH)	

MISCELLANEOUS NOTES:

1. STRUCTURAL DRAWINGS ARE INTENDED TO BE USED WITH ARCHITECTURAL AND MECHANICAL DRAWINGS. CONTRACTOR IS RESPONSIBLE FOR COORDINATING THE REQUIREMENTS OF ALL DRAWINGS INTO THEIR SHOP DRAWINGS AND WORK.
2. NO OPENINGS, OTHER THAN THOSE SHOWN ON DESIGN DRAWINGS AND APPROVED SHOP DRAWINGS, SHALL BE MADE WITHOUT THE WRITTEN APPROVAL OF THE ARCHITECT.
3. NO CHANGE IN SIZE OR DIMENSION OF STRUCTURAL MEMBERS SHALL BE MADE WITHOUT THE WRITTEN APPROVAL OF THE ARCHITECT.
4. OPENINGS OF 1'-4" AND LESS ON A SIDE ARE GENERALLY NOT SHOWN ON THE STRUCTURAL DRAWINGS. REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS FOR LOCATIONS AND DIMENSIONS OF THOSE OPENINGS. PROVIDE REINFORCING AROUND OPENINGS PER TYPICAL DETAILS SHOWN ON STRUCTURAL DRAWINGS.
5. THE CONTRACTOR IS RESPONSIBLE FOR LIMITING THE AMOUNT OF CONSTRUCTION LOAD IMPOSED UPON STRUCTURAL FRAMING. CONSTRUCTION LOADS SHALL NOT EXCEED THE CAPACITY OF THE FRAMING AT THE TIME THE LOADS ARE IMPOSED.
6. THE STRUCTURE IS DESIGNED TO FUNCTION AS A UNIT UPON COMPLETION. THE CONTRACTOR SHALL FURNISH ALL TEMPORARY BRACING AND / OR SUPPORTS REQUIRED AS THE RESULT OF THE CONTRACTOR'S CONSTRUCTION METHODS AND / OR SEQUENCES.
7. DO NOT SCALE THESE DRAWINGS, USE DIMENSIONS.
8. CONTRACTOR'S CONSTRUCTION AND / OR ERECTION SEQUENCES SHALL RECOGNIZE AND CONSIDER THE EFFECTS OF THERMAL MOVEMENTS OF STRUCTURAL ELEMENTS DURING THE CONSTRUCTION PERIOD. EXPANSION JOINTS SHOWN ON THE DRAWINGS HAVE BEEN DESIGNED TO ACCOMMODATE ANTICIPATED THERMAL MOVEMENT AFTER THE BUILDING IS COMPLETE.
9. THE CONTRACTOR SHALL INFORM THE ARCHITECT IN WRITING OF ANY DEVIATION FROM THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL NOT BE RELIEVED OF THE RESPONSIBILITY FOR SUCH DEVIATION BY THE ARCHITECTS APPROVAL OF SHOP DRAWINGS, PRODUCT DATA, ETC., UNLESS THE CONTRACTOR HAS SPECIFICALLY INFORMED THE ARCHITECT OF SUCH DEVIATION AT THE TIME OF SUBMISSION, AND THE ARCHITECT HAS GIVEN WRITTEN APPROVAL TO THE SPECIFIC DEVIATION.
10. ALL THING WHICH, IN THE OPINION OF THE CONTRACTOR, APPEAR TO BE DEFICIENCIES, OMISSIONS, CONTRADICTIONS AND AMBIGUITIES, IN THE PLANS AND SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT. PLANS AND / OR SPECIFICATIONS WILL BE CORRECTED, OR A WRITTEN INTERPRETATION OF THE ALLEGED DEFICIENCY, OMISSION, CONTRADICTION OR AMBIGUITY WILL BE MADE BY THE ARCHITECT BEFORE THE EFFECTED WORK PROCEEDS.

STATE OF ILLINOIS — JOHN L. TRANKINA — 2-18-25 — EXP. 11-26 — 4901 — LICENSED STRUCTURAL ENGINEER

(1)

		REQUIRED SPECIAL INSPECTIONS	YES	
			CONTINUOUS	PERIODIC
1		**STEEL CONSTRUCTION -IBC 1705.2**		
	A	MATERIAL VERIFICATION OF HIGH STRENGTH BOLTS, NUTS, AND WASHERS		X
	B	INSPECTION OF HIGH STRENGTH BOLTING		
		BEARING TYPE		X
		SLIP CRITICAL TYPE	x	
	C	INSPECTION OF WELDING		
		COMPLETE AND PARTIAL PEN WELDS	X	
		MULTI PASS FILLET WELDS	X	
		SINGLE PASS FILLET WELDS > 5/16	X	
		SINGLE PASS FILLET WELDS < 5/16 R		X
		FLOOR AND ROOF DECK		X
	D	INSPECTION OF STEEL FRAME DETAILS		X
	E	VERIFY DEPTH WIDTH & GAUGE OF STUDS, COMPLY WITH CONSTRUCTION DOCUMENTS		X
2		**CONCRETE CONSTRUCTION -IBC 1705.3**		
	A	INSPECTION OF REINFORCING STEEL		X
	B	INSPECTION OF BOLTS INSTALLED IN CONCRETE	X	
	C	VERIFY USE OF REQUIRED MIX DESIGN		X
	D	SAMPLING FRESH CONCRETE AND PERFORMING SLUMP, AIR CONTENT AND DETERMINING THE TEMPERATURE OF FRESH CONCRETE AT TIME OF MAKING SPECIMENS FOR STRENGTH TESTS.	X	
	E	INSPECTION OF CONCRETE PLACEMENT	X	
	F	INSPECTION OF FOR MAINTENANCE OF SPECIFIED CURING TECHNIQUES		X
3		**MASONRY CONSTRUCTION -IBC 1705.4**		
	A	AS MASONRY CONSTRUCTION BEGINS:		
		PROPORTIONS OF SITE PREPARED MORTAR		X
		CONSTRUCTIONS OF MORTAR JOINTS		X
		LOCATIONS OF REINFORCING AND CONNECTORS		X
	B	INSPECTION PROGRAM:		
		SIZE AND LOCATION OF STRUCTURAL ELEMENTS		X
		TYPE, SIZE, AND LOCATION OF ANCHORS		X
		SIZE, GRADE, AND TYPE OF REINFORCEMENT		X
		WELDING OF REINFORCING BARS		N/A
		PROTECTION OF MASONRY FOR HOT OR COLD WEATHER		X
	C	PRIOR TO GROUTING		
		GROUT SPACE IS CLEAN		X
		PLACEMENT OF REINFORCEMENT		X
		PROPORTIONS OF SITE-PREPARED GROUT		X
		CONSTRUCTION OF MORTAR JOINTS		X
	D	GROUT PLACEMENT VERIFIED TO ENSURE COMPLIANCE WITH CODE AND CONSTRUCTION DOCUMENTS	X	
	E	PREPARATION OF GROUT SPECIMENS, MORTAR SPECIMENS AND/OR PRISMS SHALL BE OBSERVED	X	
	F	COMPLIANCE WITH REQUIRED INSPECTION PROVISIONS OF CONSTRUCTION DOCUMENTS AND THE APPROVED SUBMITTALS SHALL BE VERIFIED		X
4		**SOILS -IBC 1705.6**		
	A	VERIFY SITE HAS BEEN PREPARED IN ACCORDANCE WITH APPROVED SOILS REPORT		X
	B	FILL PLACEMENT - VERIFY MATERIAL, MAXIMUM LIFT THICKNESS AND COMPACTION OF FILL ARE IN ACCORDANCE WITH THE SOILS REPORT		X
	C	VERIFY IN PLACE DRY DENSITY OF FILL COMPLIES WITH SOILS REPORT		X
	D	CONFIRM ALLOWABLE SOIL BEARING PRESSURE COMPLIES WITH SOILS REPORT	X	

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(1) 12/18/2024

REVISIONS

THIS DRAWING		
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PROPOSED
AVANT GARDENS - BANQUET HALL
370 VETERANS PARKWAY, NEW LENOX, ILLINOIS
FOR: UNLIMITED MASONRY AND CONSTRUCTION, INC.

SHEET TITLE
GENERAL NOTES

2428

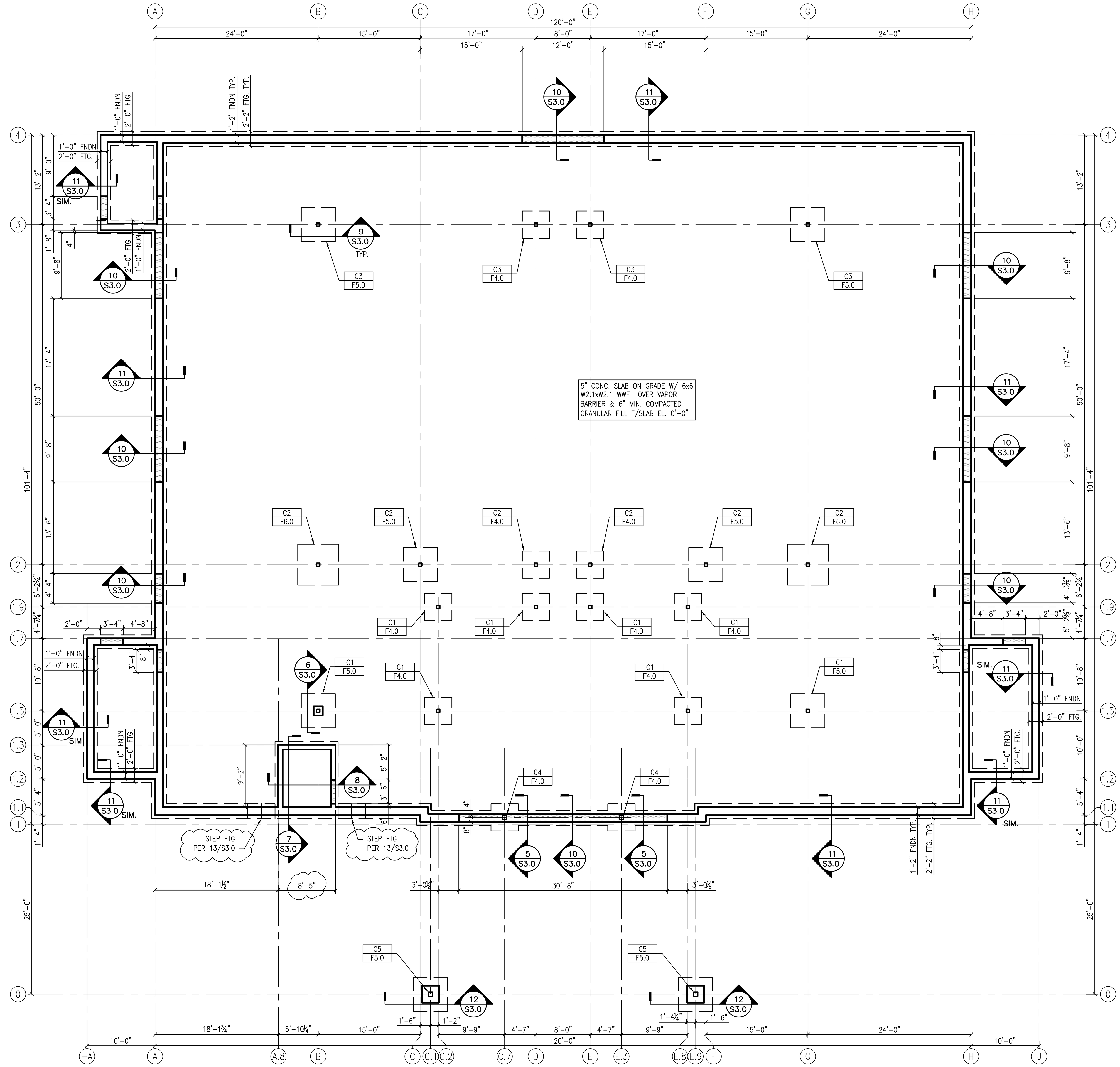
S 1.0

COLUMN SCHEDULE

MARK	SIZE	BASE PLATE	ANCHORS	CAP PLATE
C1	HSS 4x4x¼	¾"x10"x10"	(4) ¾"øx12"+3" HOOK	¼"
C2	HSS 6x6x¼	¾"x12"x12"	(4) ¾"øx12"+3" HOOK	¼"
C3	HSS 6x6x5⁄16	¾"x12"x12"	(4) ¾"øx12"+3" HOOK	¼"
C4	HSS 8x8x⅜	¾"x14"x10"	(4) ¾"øx12"+3" HOOK	¼"
C5	HSS 8x8x⅜	1"x14"x14"	(4) ¾"øx12"+3" HOOK	¼"

FOOTING SCHEDULE

MARK	SIZE	REINFORCING
F4.0	4'–0" x 4'–0" x 1'–0"	(4) #5 EACH WAY AT BOTTOM
F5.0	5"–0" x 5'–0" x 1'–0"	(5) #5 EACH WAY AT BOTTOM
F6.0	6"–0" x 6'–0" x 1'–0"	(6) #5 EACH WAY AT BOTTOM



1 FOUNDATION PLAN

SCALE: 1/8"=1'-0"

- VERIFY AND COORDINATE ALL DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS.

INDICATES DEPRESSED T/FOUNDATION WALL AT DOORWAYS. SEE 10/S3.0

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STATE OF ILLINOIS
JOHN L. TRANKINA
2-18-25
EXP. 11-26
4901
LICENSED STRUCTURAL ENGINEER

△ 12/18/2024

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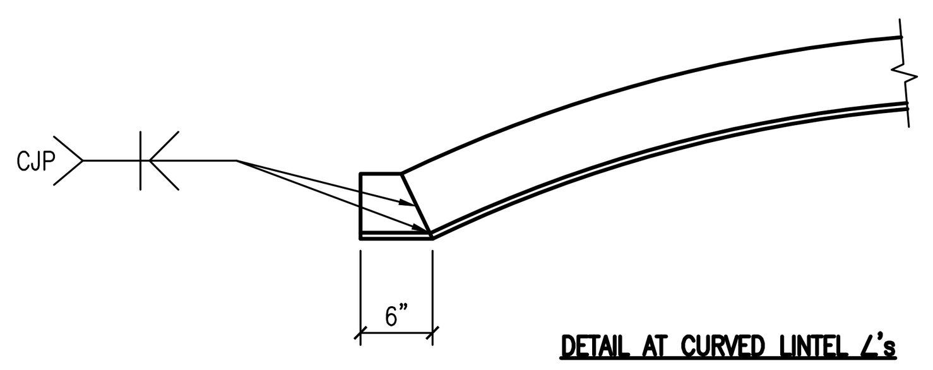
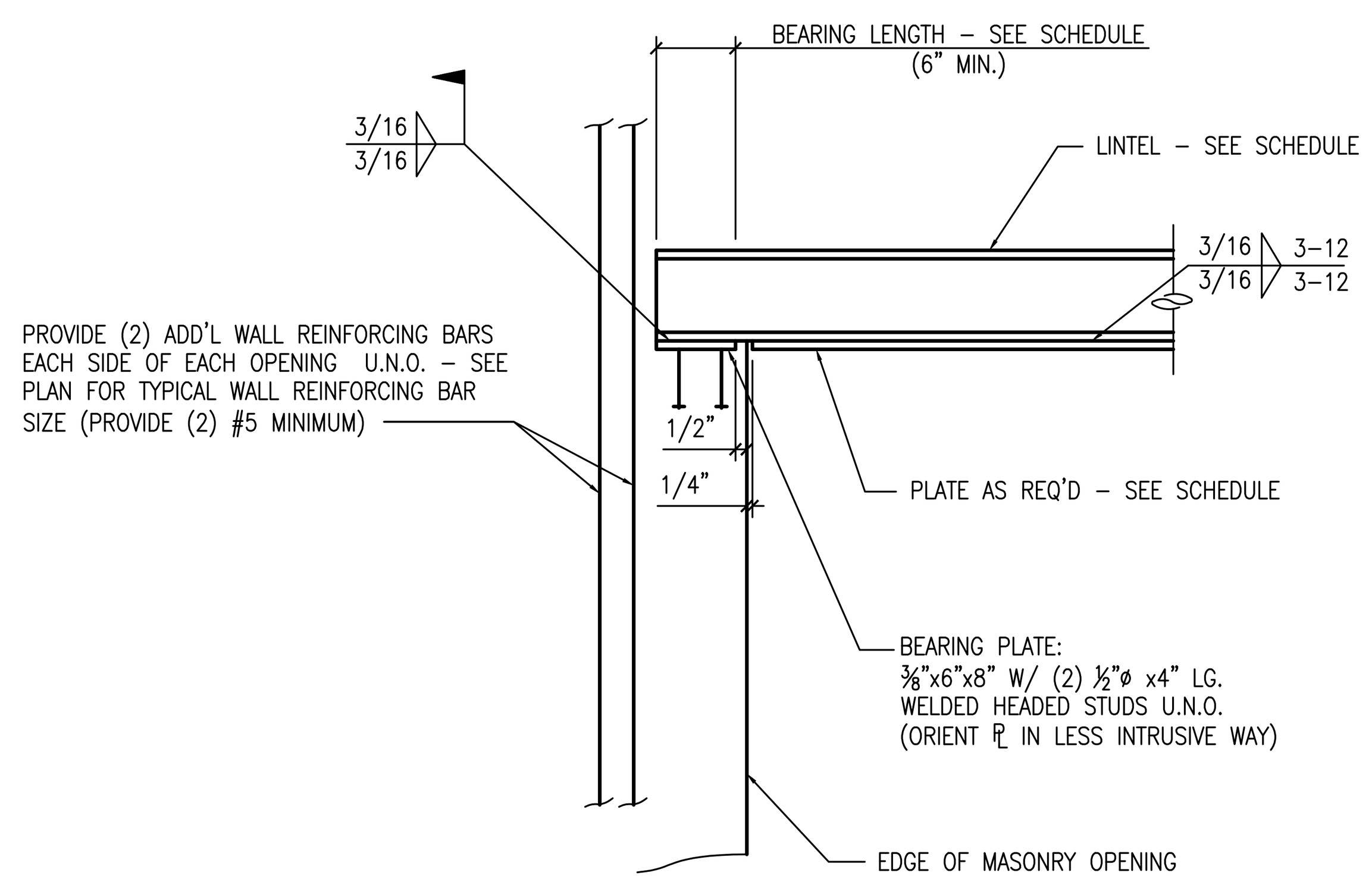
SHEET TITLE
FOUNDATION PLAN

2428

S 2.0



DETAIL AT CURVED LINTEL ∠'s

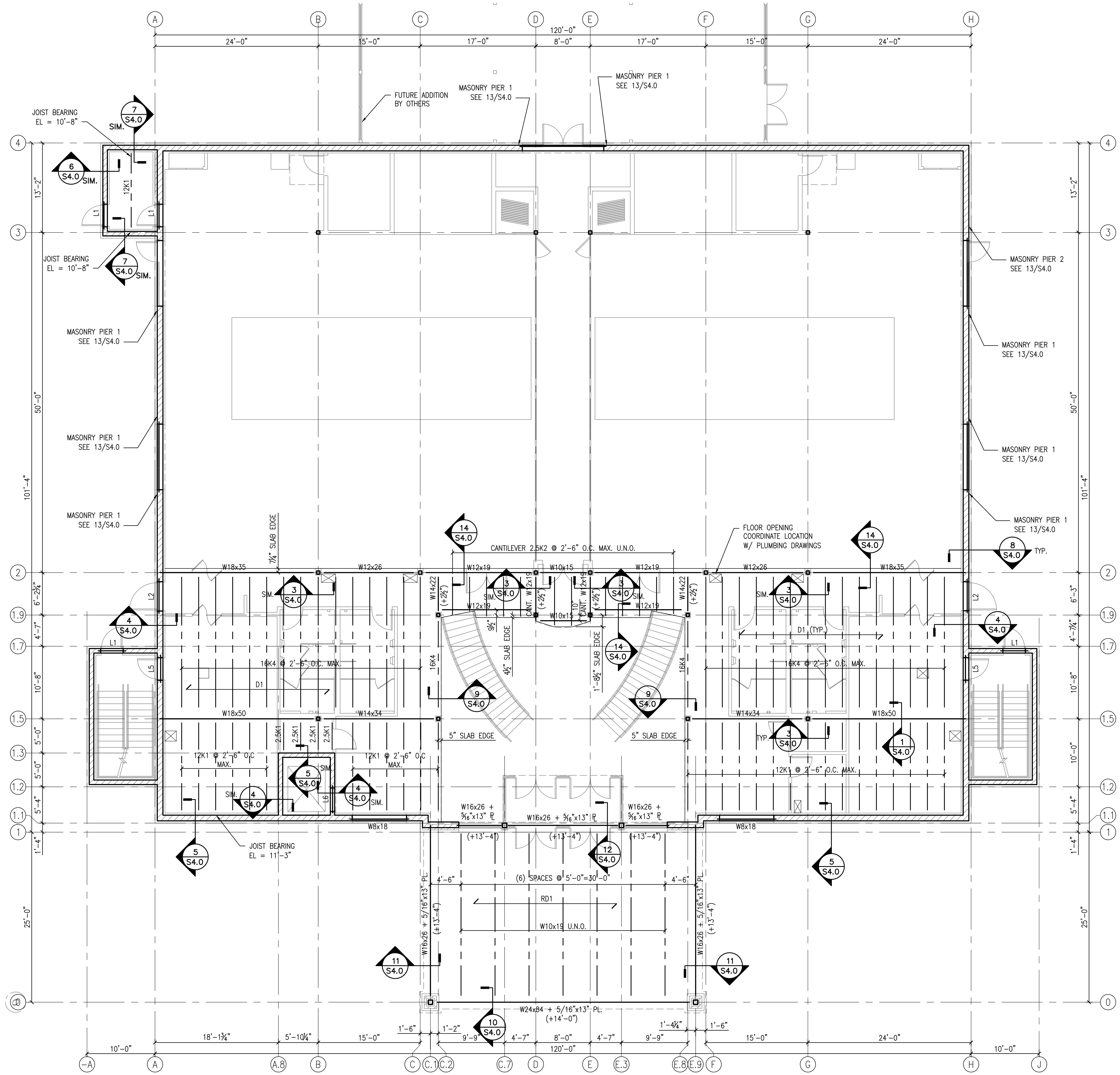


2 TYPICAL LINTEL DETAIL

SCALE: N.T.S.

LINTEL SCHEDULE

MARK	SECTION	SHAPE	BEARING LENGTH	NOTES
L1	(3) ∠3½x3½x5⁄16	∠	8"	
L2/L2A	∠3½x3½x5⁄16 + ∠4x4x3⁄8 + ∠5x5x3⁄8	∠	8"	CURVED AT ∠2A
L3	CURVED W8x21 + CONT. 5⁄16x13" PL.	W–SHAPE	8"	
L4	CURVED W16x26 + CONT. 5⁄16x13" PL.	W–SHAPE	8"	
L5	∠4x4x5⁄16 + ∠5x5x5⁄16	∠	8"	
L6	(2) ∠3½x3½x5⁄16	∠	8"	



1 SECOND FLOOR FRAMING PLAN

SCALE: 1/8"=1'–0"

- VERIFY AND COORDINATE ALL DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS.
- D1: 0.6 26GA FORM DECK + 2" N.W. CONCRETE TOPPING (2½" TOTAL) W/ 6x6–W2.1xW2.1 WWF. T/CONCRETE = 11'–8"
- NOTE 'A': MECHANICAL CHASE CLOSETS ARE SHOWN FOR REFERENCE AND ONLY INDICATE THE EXTENT OF DECK AND SLAB OPENING. (BEAMS AND JOISTS SHALL REMAIN) GENERAL CONTRACTOR SHALL COORDINATE OTHER OPENINGS NOT SHOWN TO FIT IN BETWEEN JOIST SPACES.
- T/STEEL ELEV.=11'–3"
- SEE S2.0 FOR COLUMN AND FOOTING SCHEDULES.
- PROVIDE JOIST AT ALL COLUMN LOCATIONS U.N.O.

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1 12/18/2024

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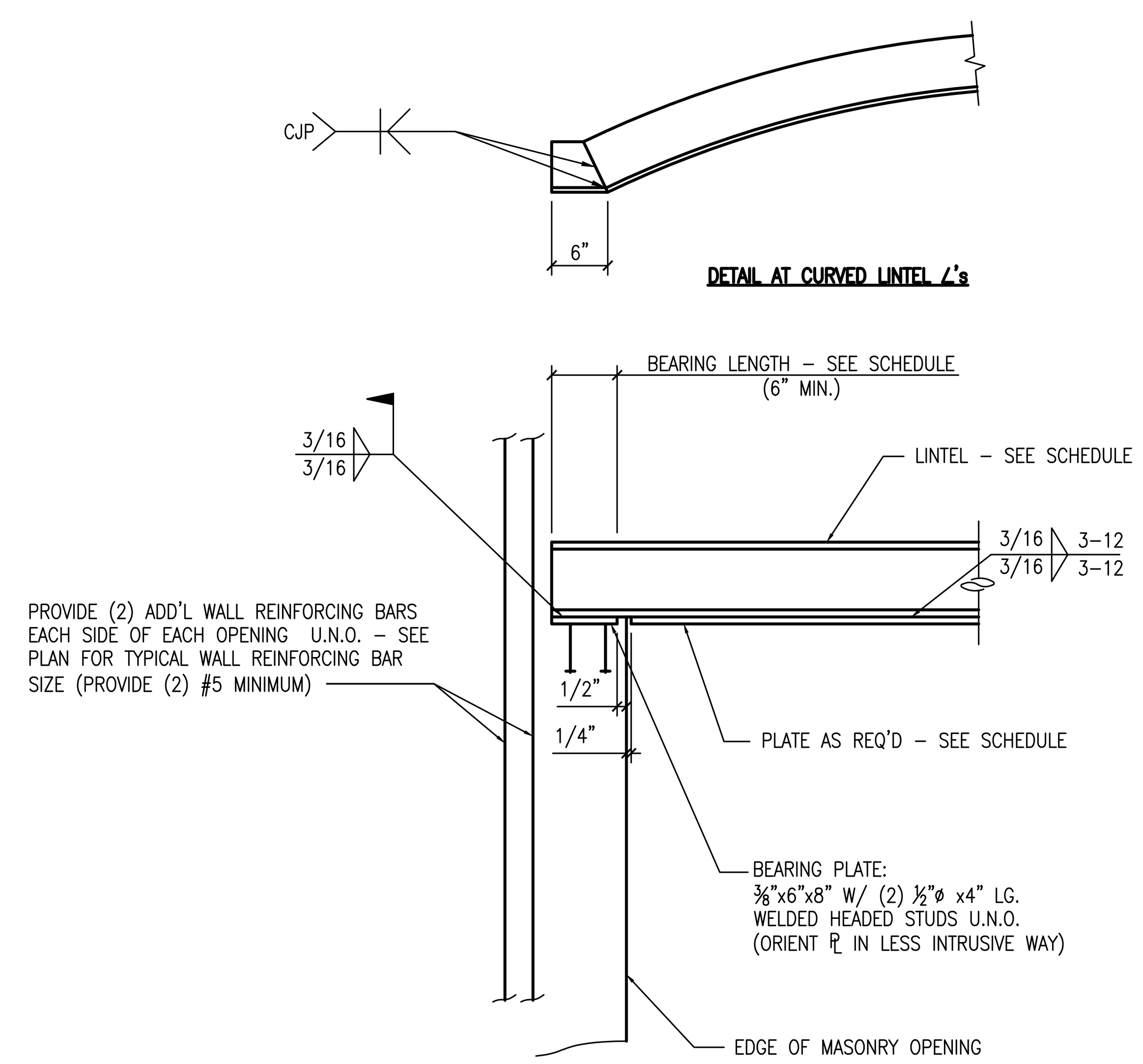
SHEET TITLE
SECOND FLOOR FRAMING PLAN

2428

S 2.1

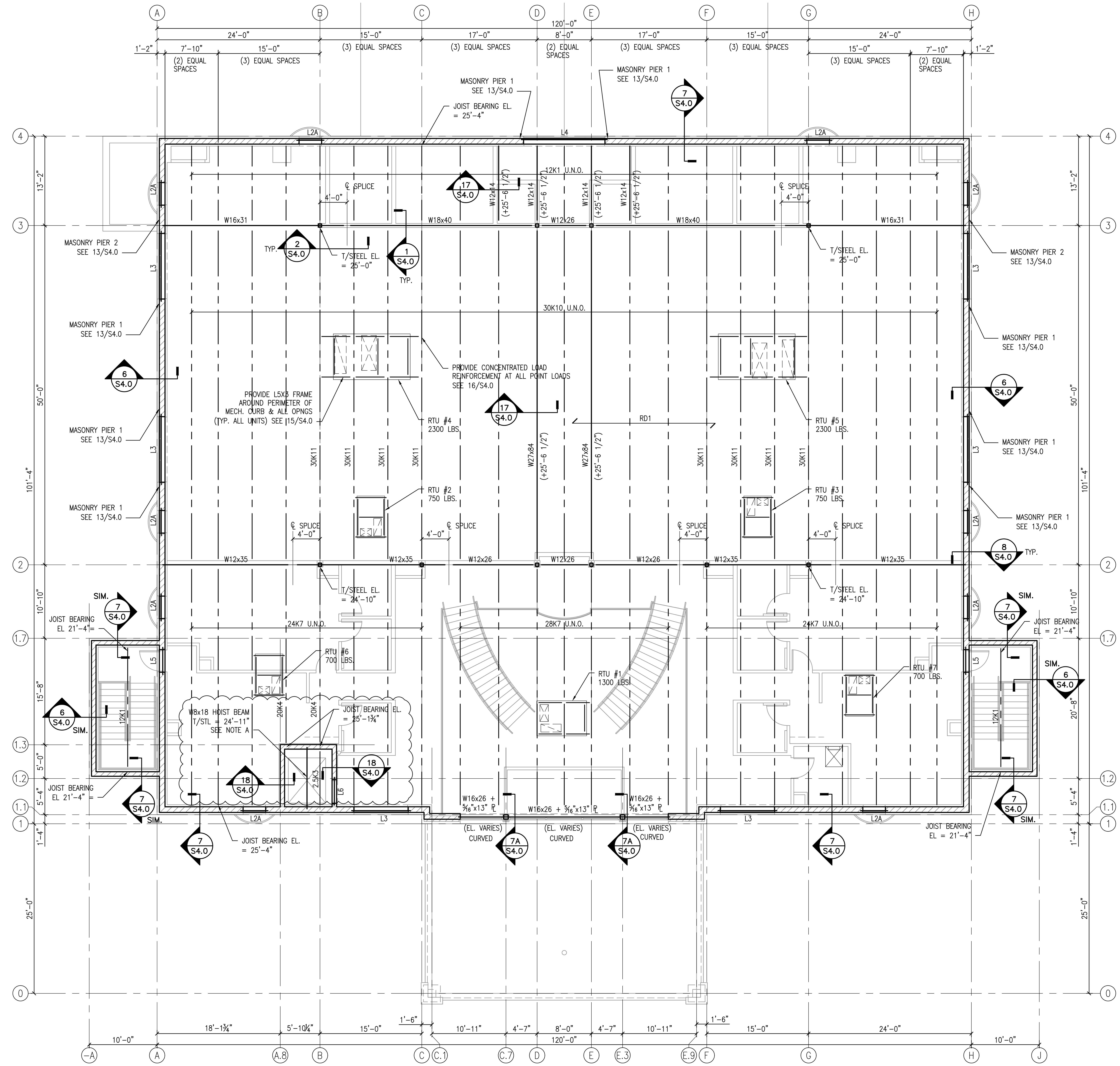


2 TYPICAL LINTEL DETAIL

SCALE: N.T.S.

LINTEL SCHEDULE

MARK	SECTION	SHAPE	BEARING LENGTH	NOTES
L1	(3) ∠3½x3½x5/16	∠	8"	
L2/L2A	∠3½x3½x5/16 + ∠4x4x3/8 + ∠5x5x3/8	∠	8"	CURVED AT ∠2A
L3	CURVED W8x21 + CONT. 5/16x13" PL.	W-SHAPE	8"	
L4	CURVED W16x26 + CONT. 5/16x13" PL.	W-SHAPE	8"	
L5	∠4x4x5/16 + ∠5x5x5/16	∠	8"	
L6	(2) ∠3½x3½x5/16	∠	8"	



1 ROOF FRAMING PLAN

SCALE: 1/8"=1'-0"

- VERIFY AND COORDINATE ALL DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS.
- RD1: PROVIDE 1½"-20 GA. WIDE RIB PAINTED METAL DECK CONTINUOUS OVER 3 SPANS. WELD TO SUPPORTS IN 36/4 PATTERN AND PROVIDE (2) #10 TEK SCREWS AT SIDE LAPS.
- SEE S2.0 FOR COLUMN & FOOTING SCHEDULES.
- PROVIDE JOIST AT ALL COLUMN LOCATIONS U.N.O.
- T/STEEL = 25'-4" UNLESS NOTED OTHERWISE.
- NOTE A: VERIFY HOIST BEAM ELEVATION W/ ELEVATOR MANUFACTURER.

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STATE OF ILLINOIS JOHN L. TRANKINA 2-18-25 EXP. 11-26 4901 LICENSED STRUCTURAL ENGINEER

1 12/18/2024

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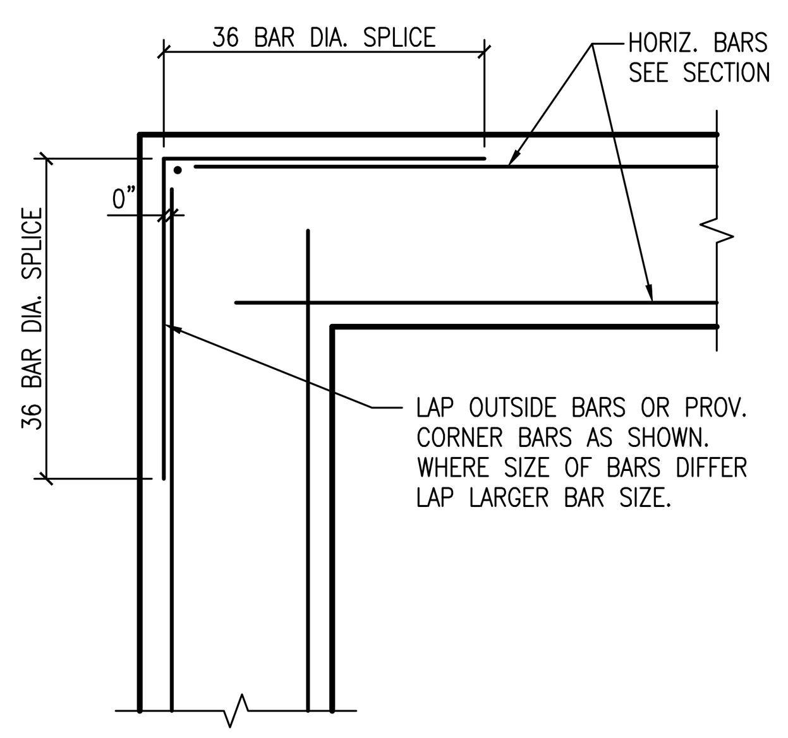
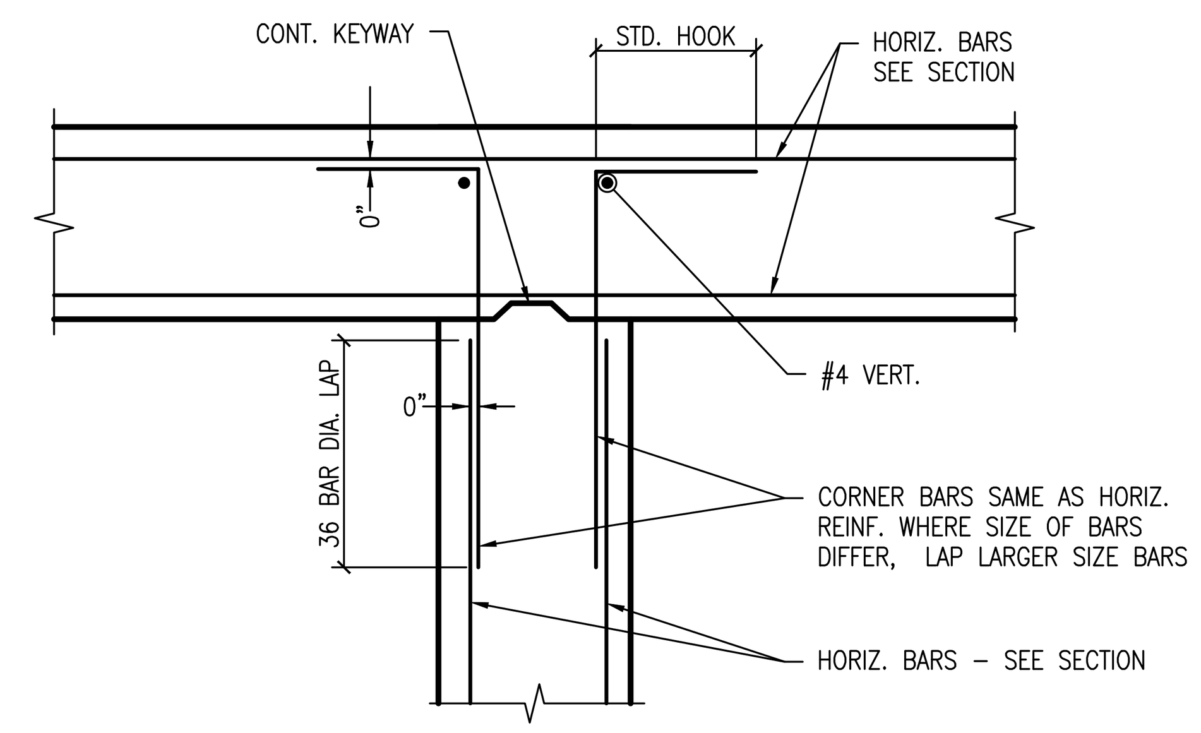
SHEET TITLE
ROOF FRAMING PLAN

2428

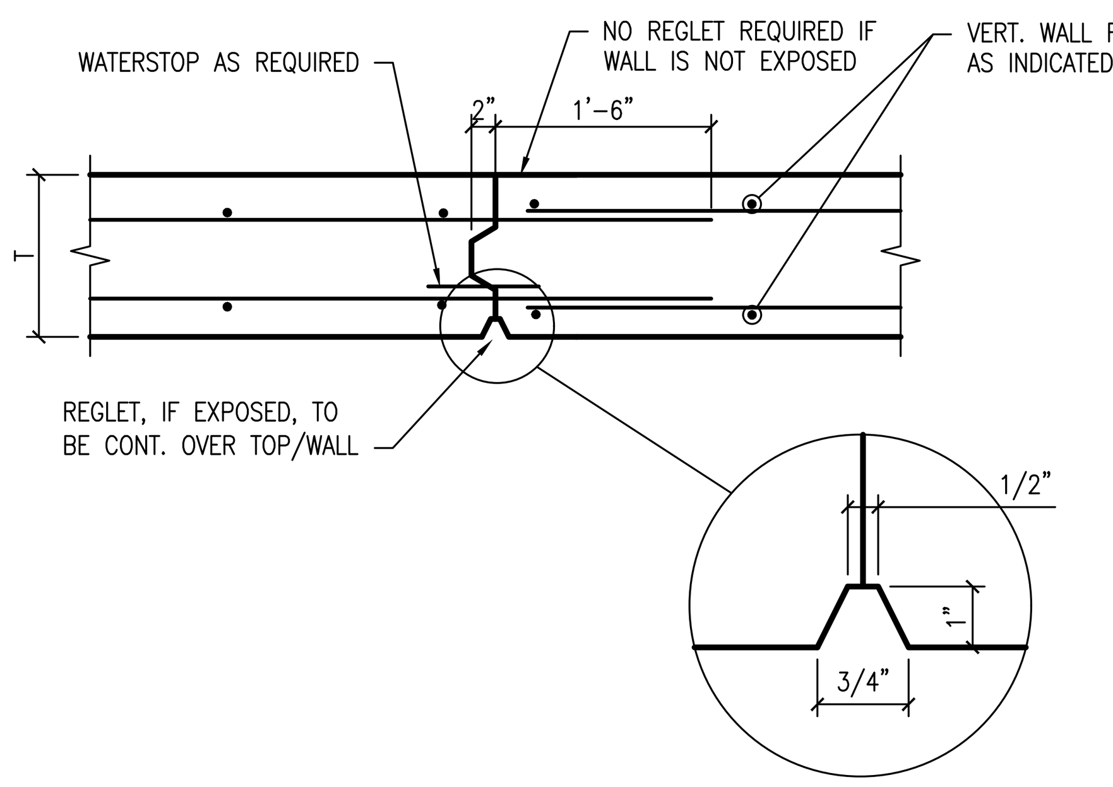
S 2.2



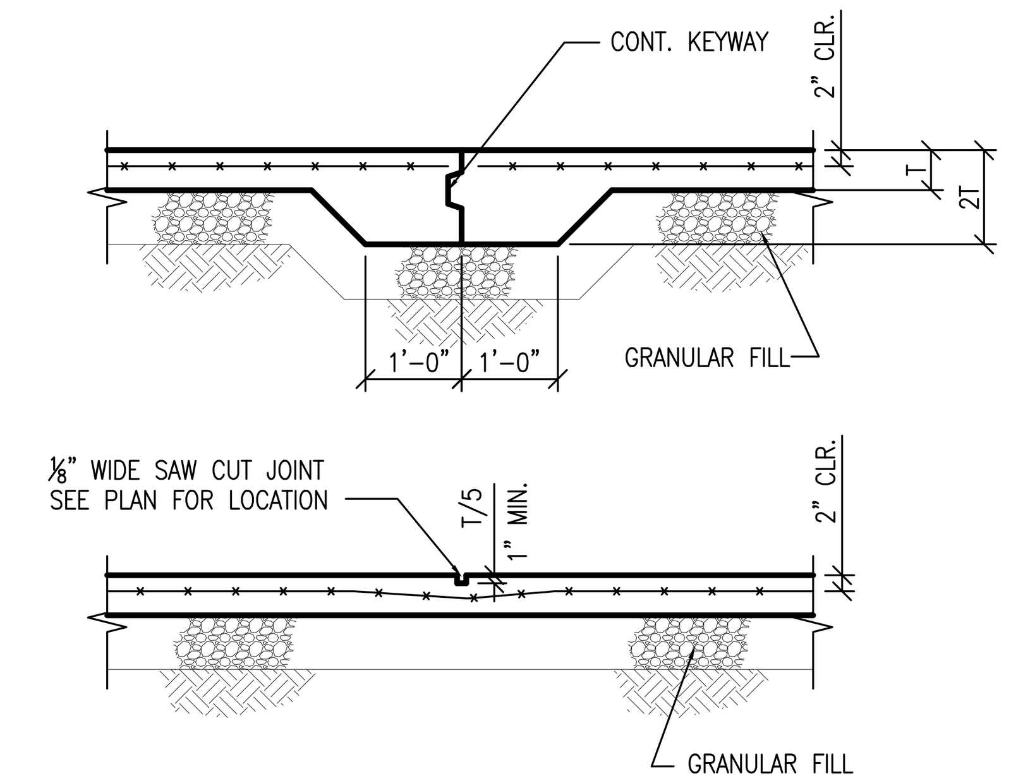
1 CORNER BAR AT CONC. WALLS
SCALE: NTS



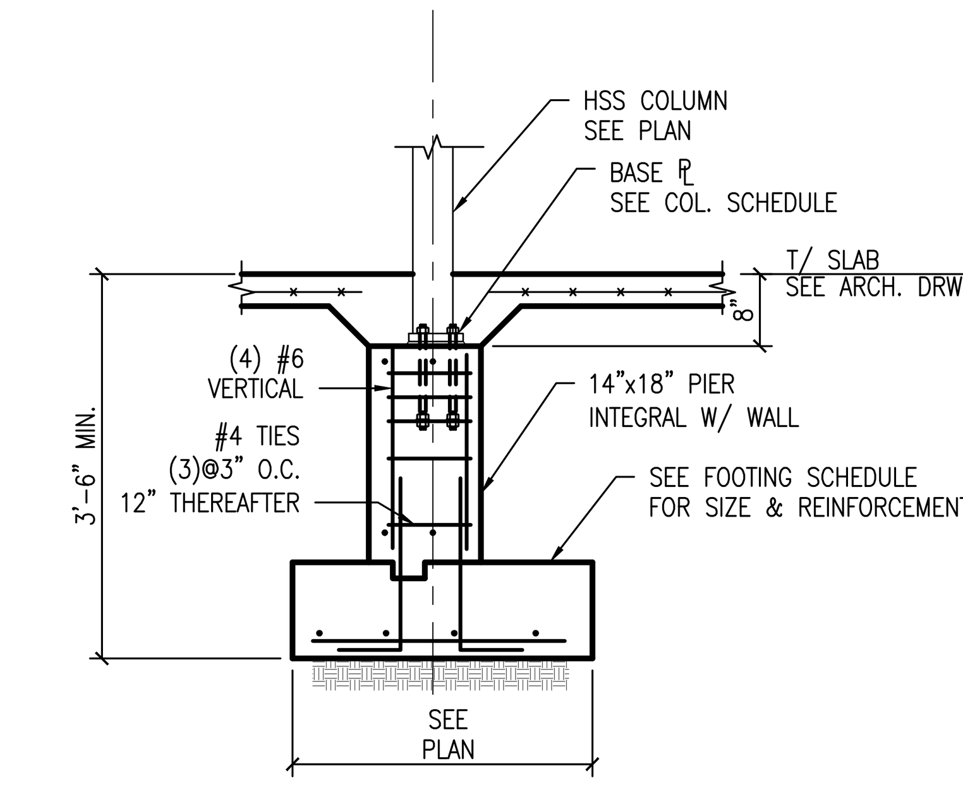
2 HORIZ. REINF. AT INTERSECTION OF CONC. WALLS
SCALE: NTS



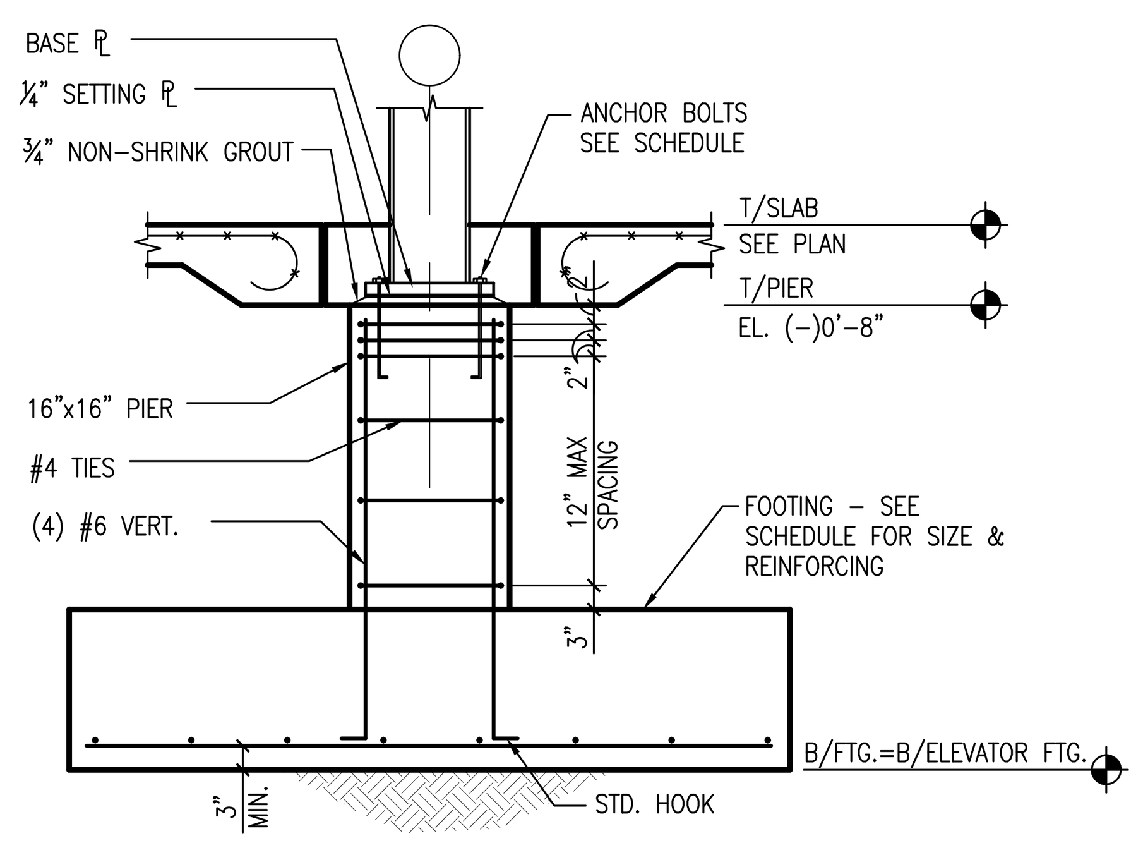
3 TYP. WALL CONSTRUCTION JOINT DETAIL
SCALE: NTS



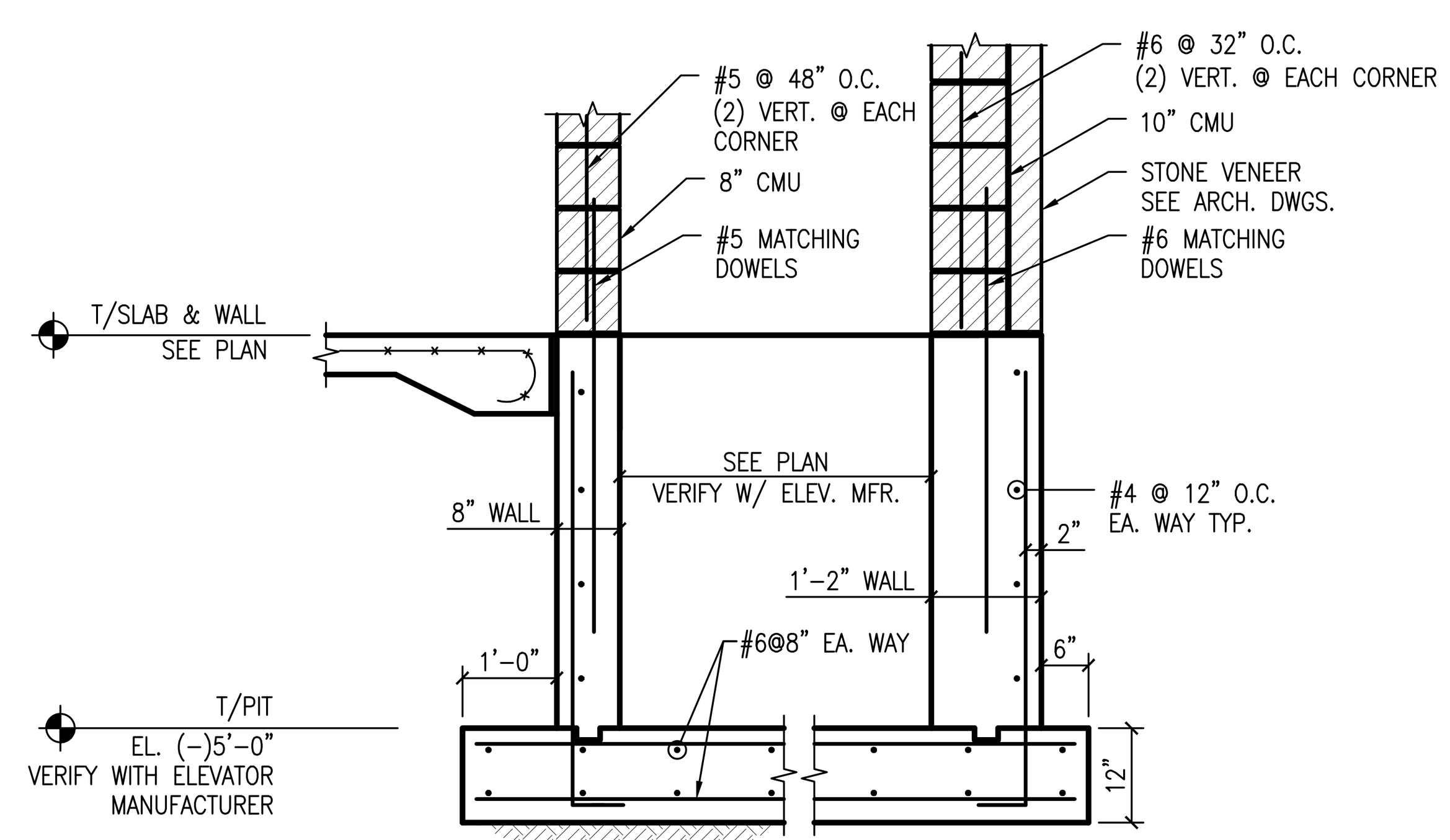
4 TYPICAL SLAB DETAIL
SCALE: NTS



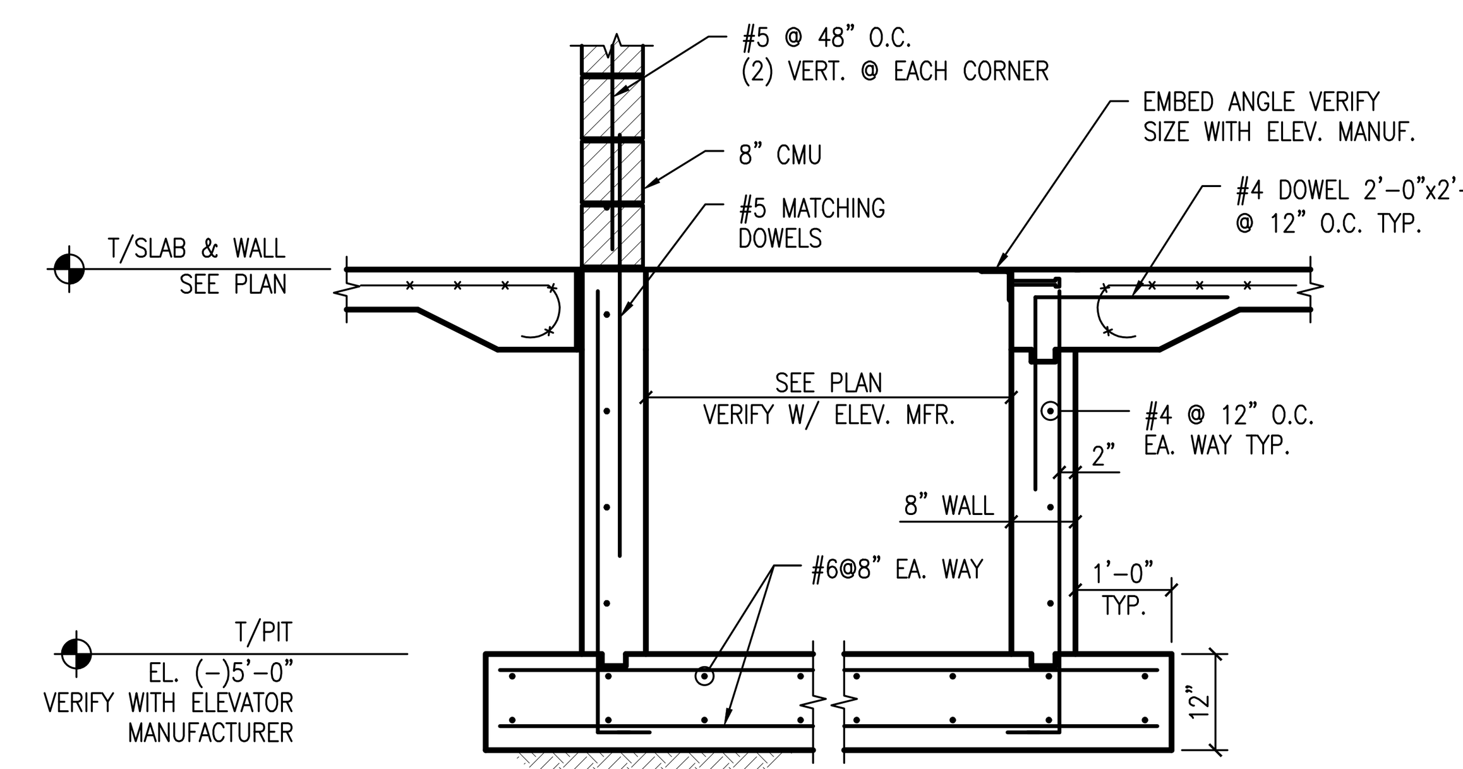
5 SECTION
SCALE: 1/2"=1'-0"



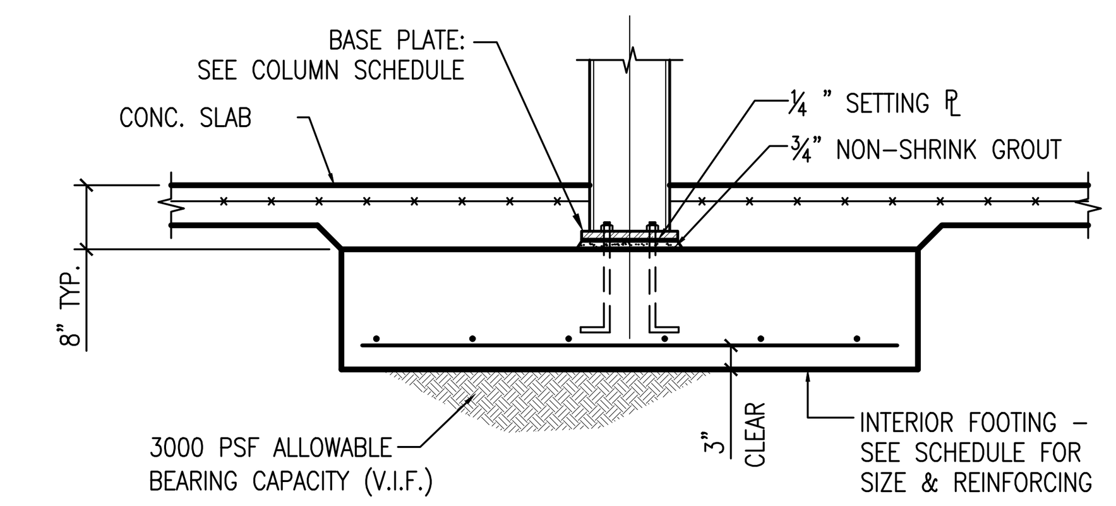
6 SECTION
SCALE: 1/2"=1'-0"



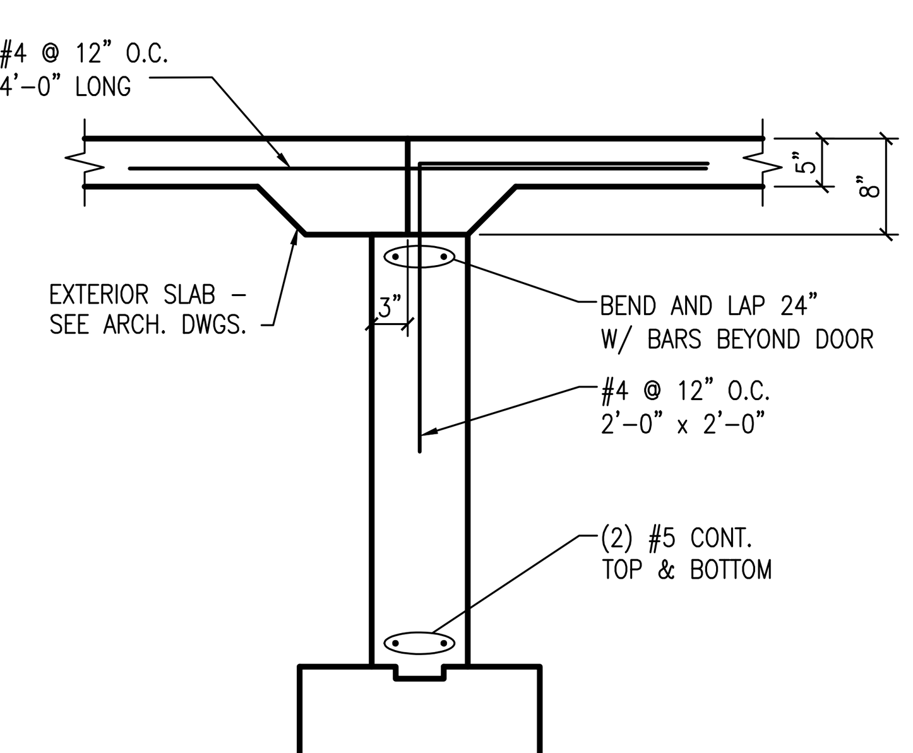
7 ELEVATOR PIT DETAIL
SCALE: 1/2"=1'-0"



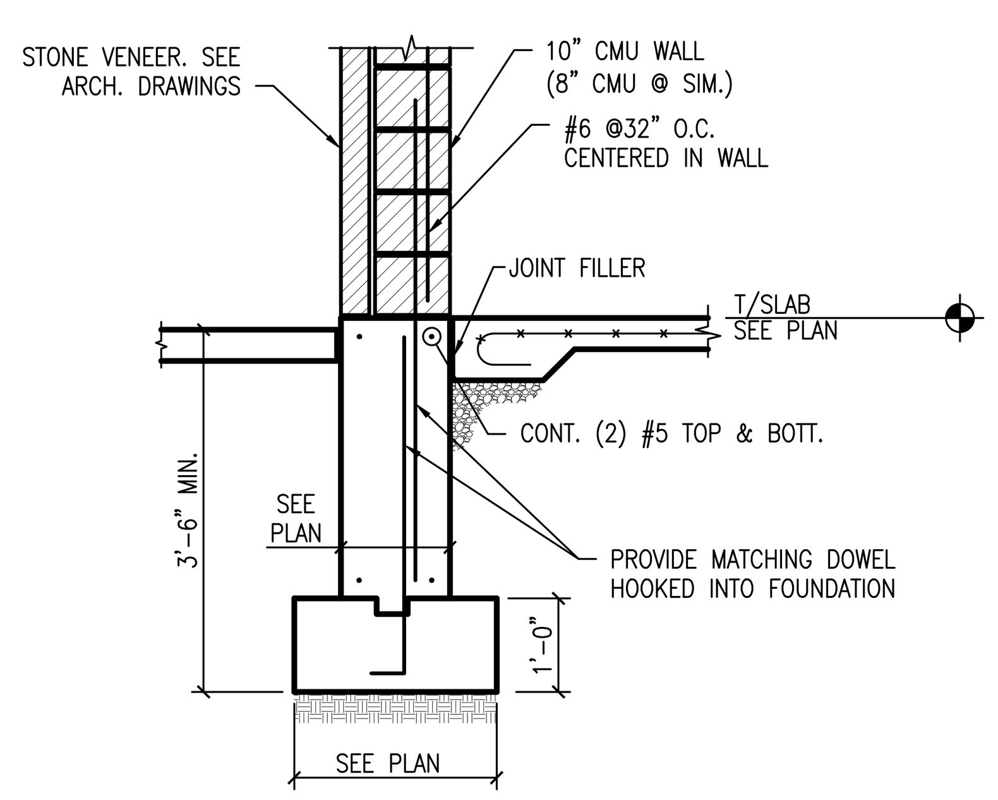
8 ELEVATOR PIT DETAIL
SCALE: 1/2"=1'-0"



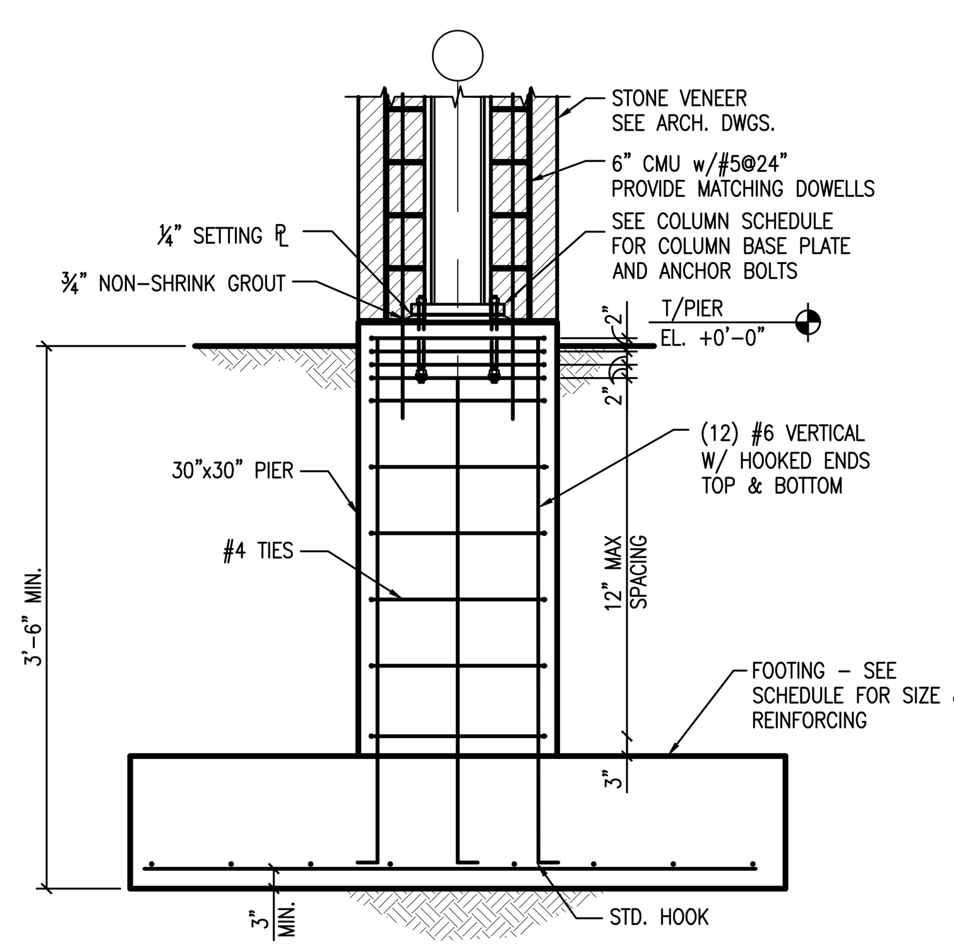
9 SECTION
SCALE: 1/2"=1'-0"



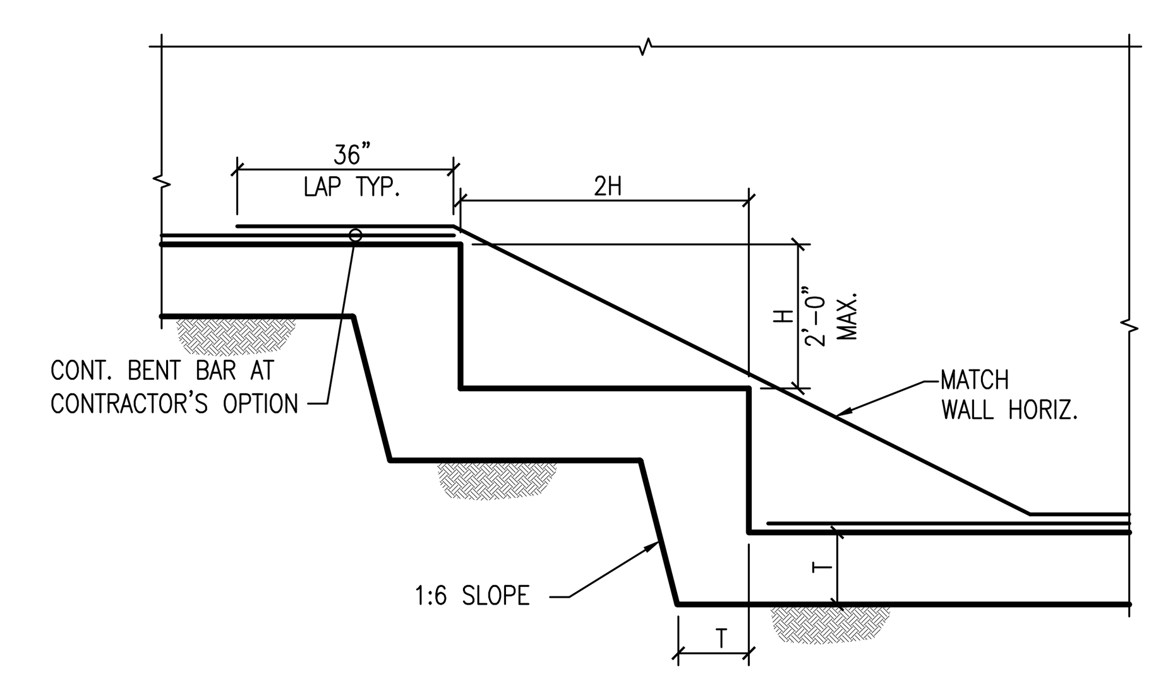
10 TYPICAL FNDN WALL AT DOORWAY
SCALE: NTS



11 SECTION
SCALE: 1/2"=1'-0"



12 SECTION
SCALE: 1/2"=1'-0"



13 TYPICAL STEP FOOTING DETAIL
SCALE: NTS

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STATE OF ILLINOIS JOHN L. TRANKINA 2-18-25 EXP. 11-26 4901 LICENSED STRUCTURAL ENGINEER

△ 12/18/2024
REVISIONS

THIS DRAWING	
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NOT	FOR CONSTRUCTION

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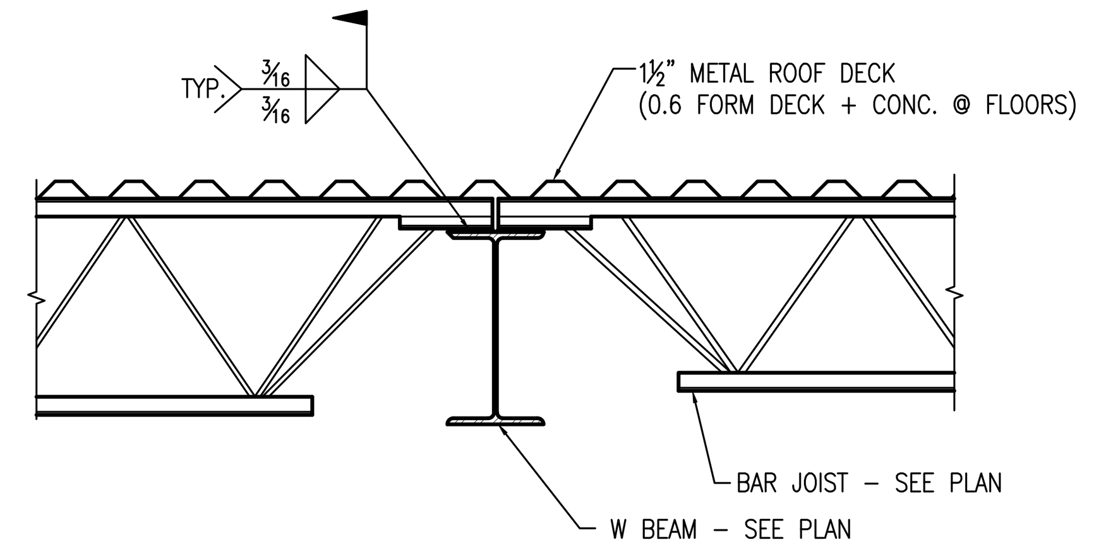
PROPOSED
AVANT GARDENS – BANQUET HALL
370 VETERANS PARKWAY, NEW LENOX, ILLINOIS
FOR: UNLIMITED MASONRY AND CONSTRUCTION, INC.

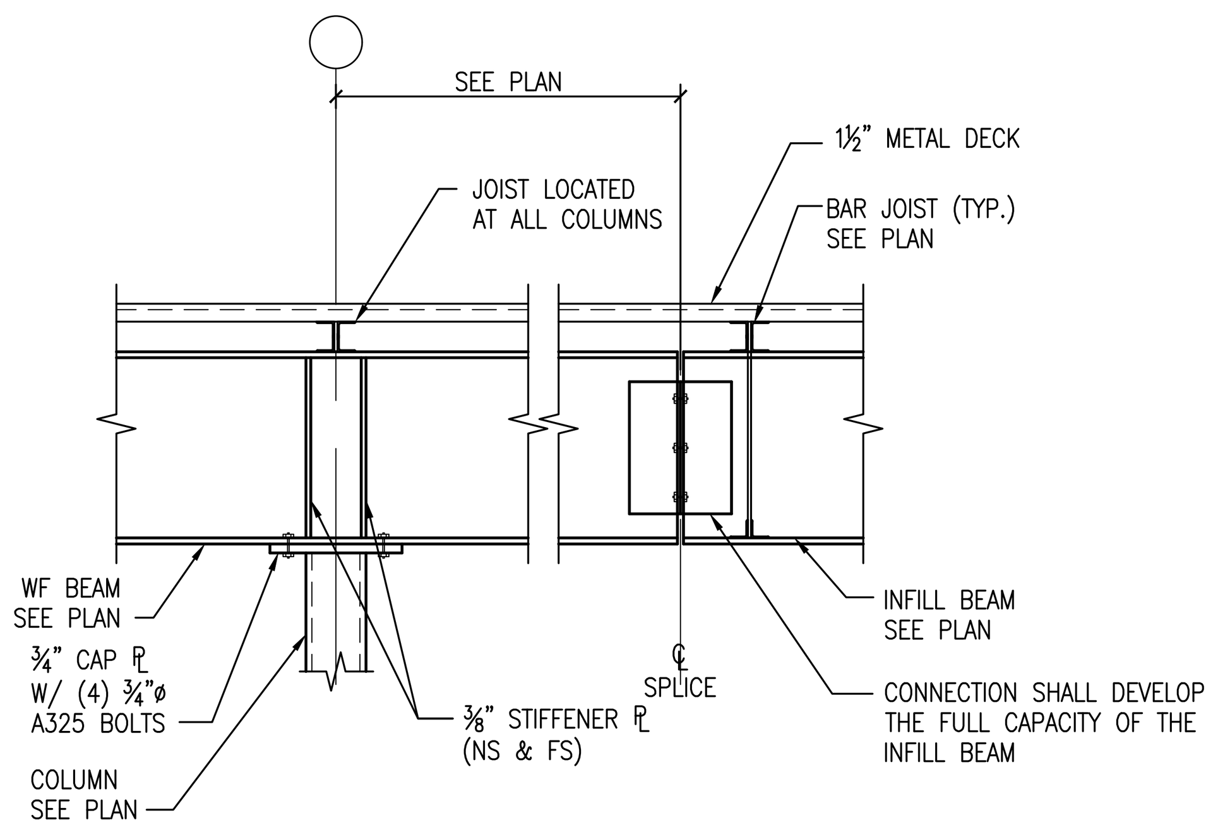
SHEET TITLE
FOUNDATION DETAILS

2428

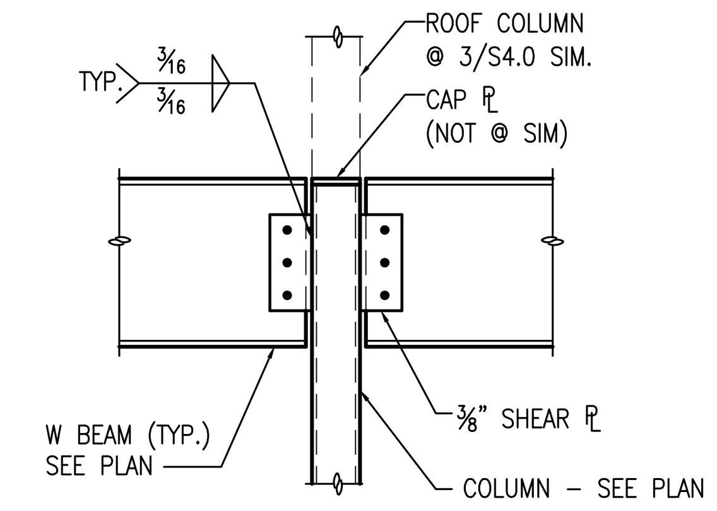
S 3.0



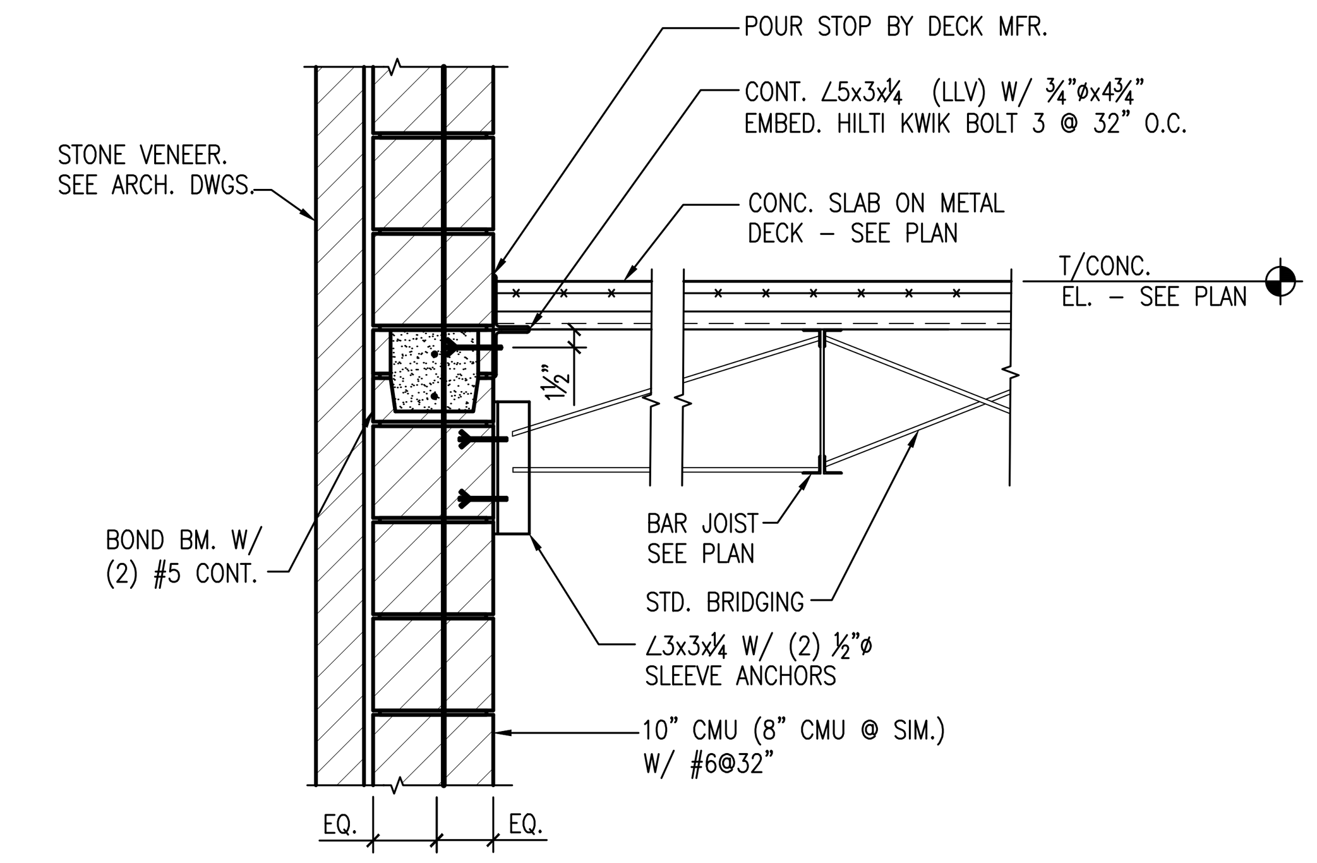
1 TYPICAL JOIST/BEAM CONNECTION
SCALE: 3/4"=1'-0"



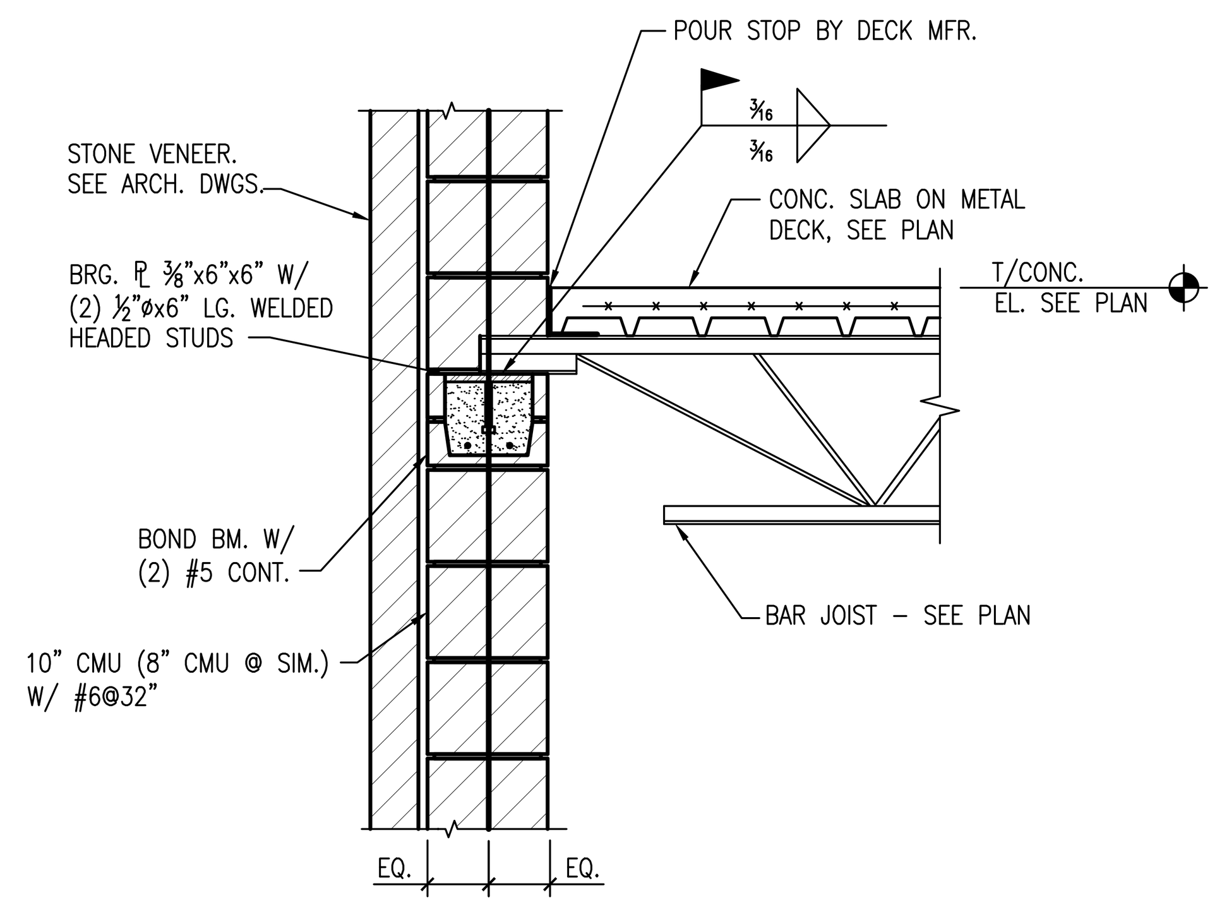
2 TYPICAL ROOF/BEAM/COLUMN CONNECTION
SCALE: 3/4"=1'-0"



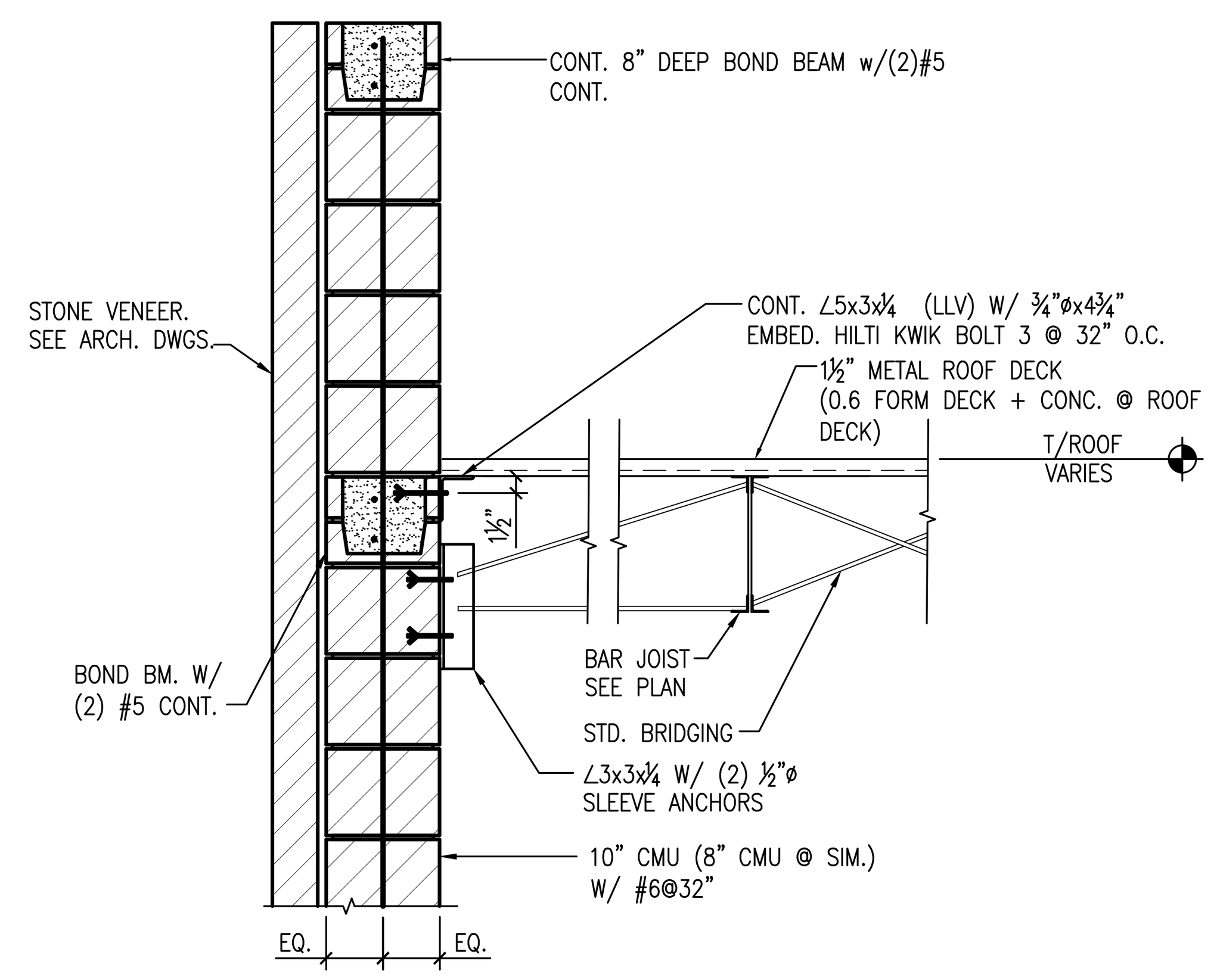
3 TYPICAL MEZZANINE BEAM CONNECTION
SCALE: 3/4"=1'-0"



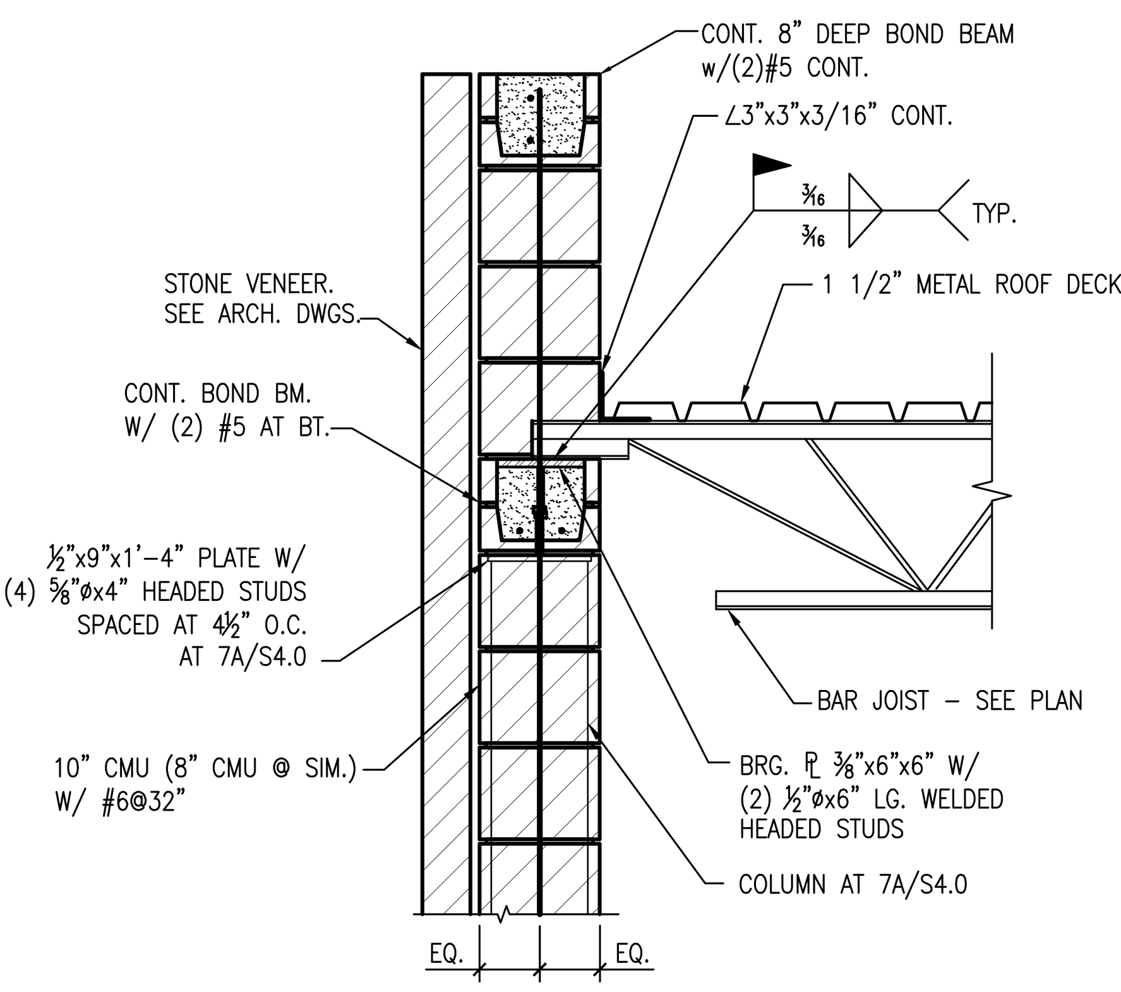
4 SECTION
SCALE: 3/4"=1'-0"



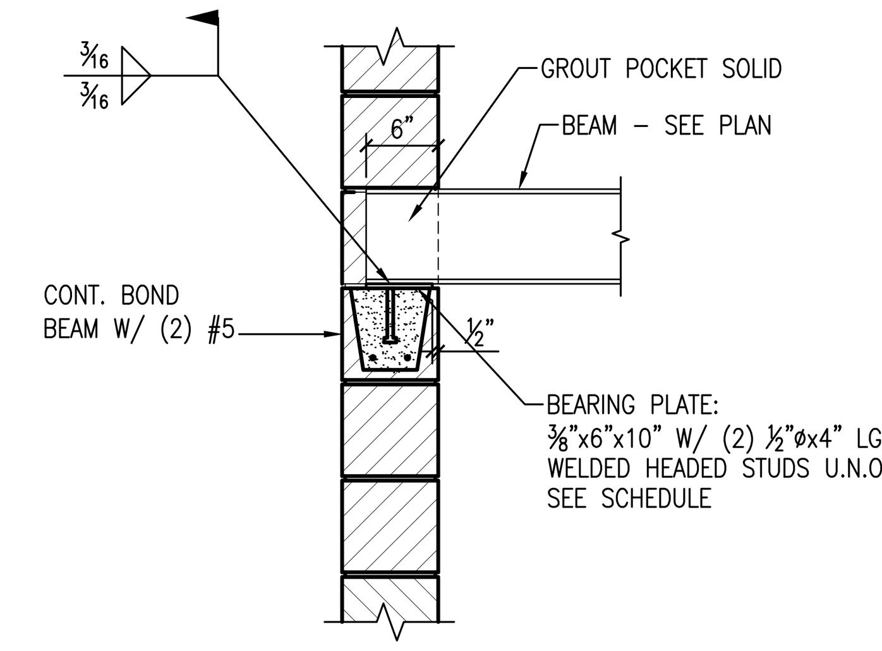
5 FLOOR BAR JOIST BEARING ON MASONRY
SCALE: 3/4"=1'-0"



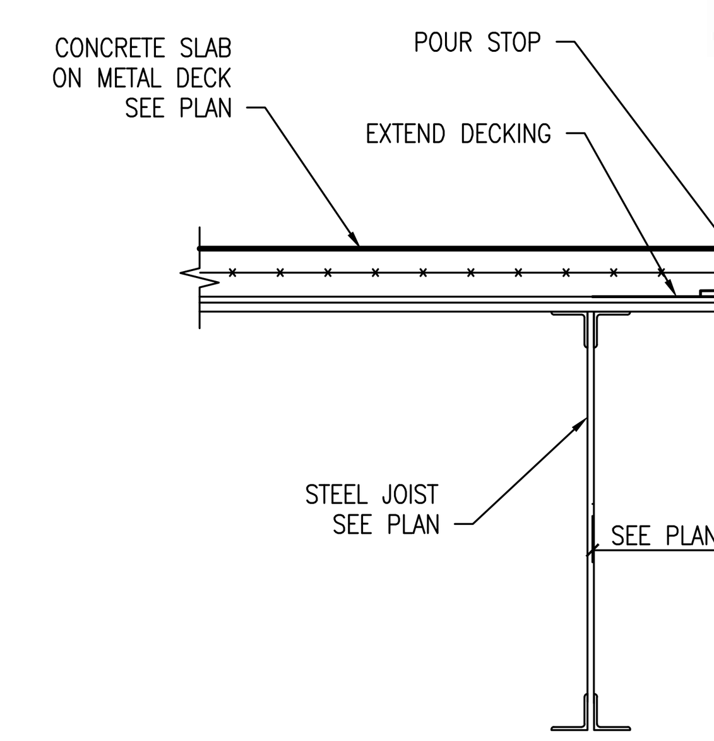
6 SECTION
SCALE: 3/4"=1'-0"



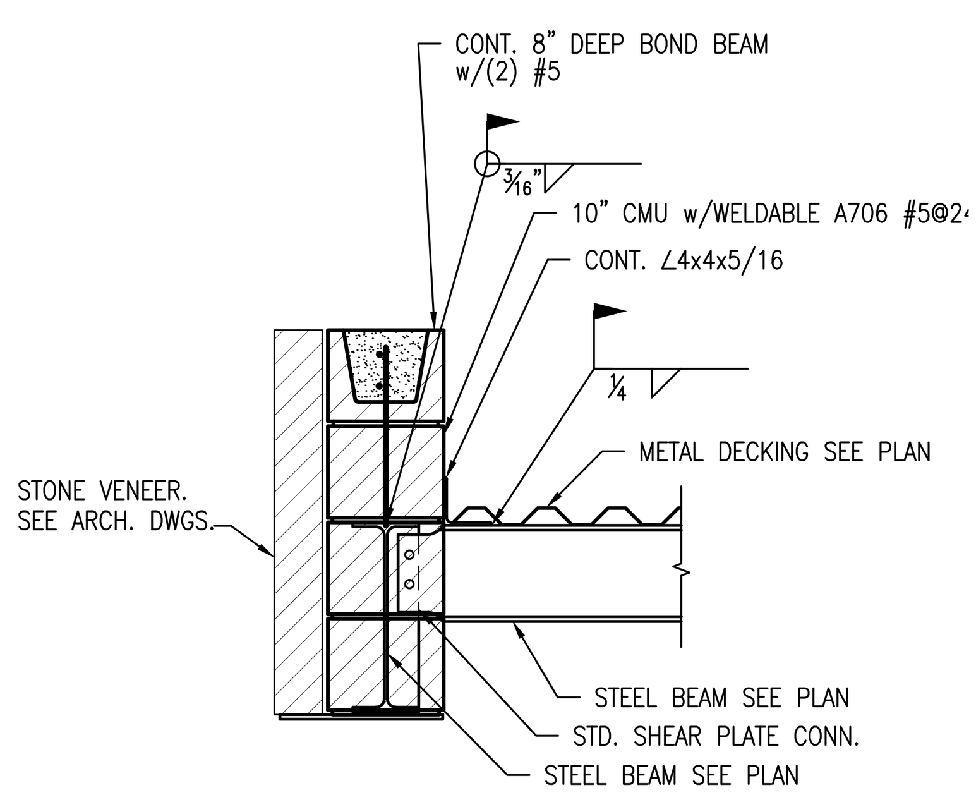
7 7A SECTION
SCALE: 3/4"=1'-0"



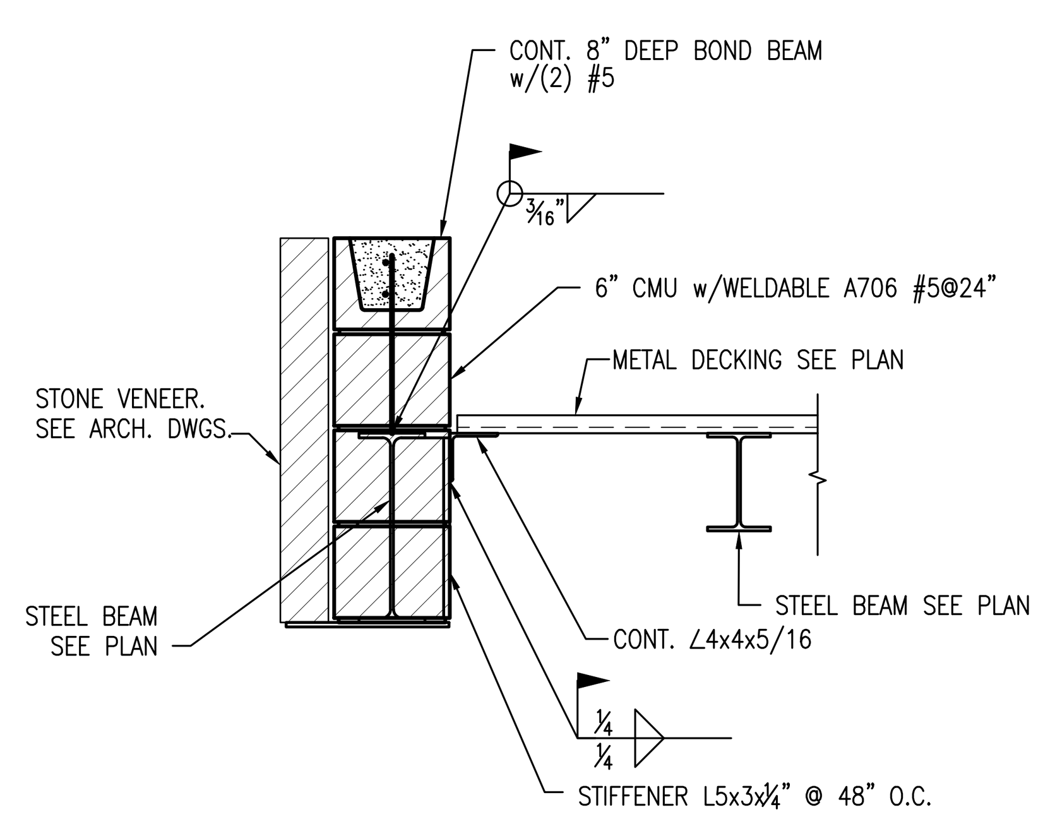
8 SECTION
SCALE: 3/4"=1'-0"



9 SECTION
SCALE: 1 1/2"=1'-0"



10 SECTION
SCALE: 3/4"=1'-0"



11 SECTION
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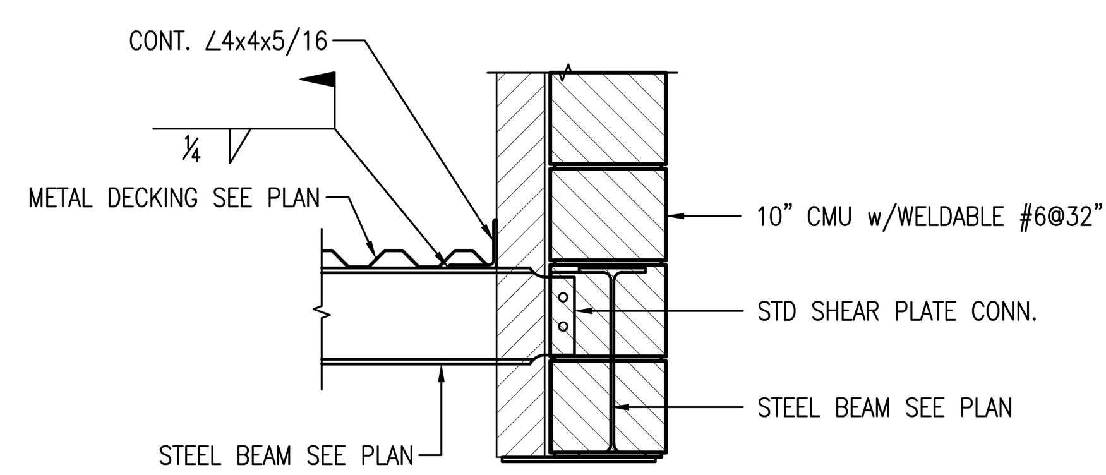
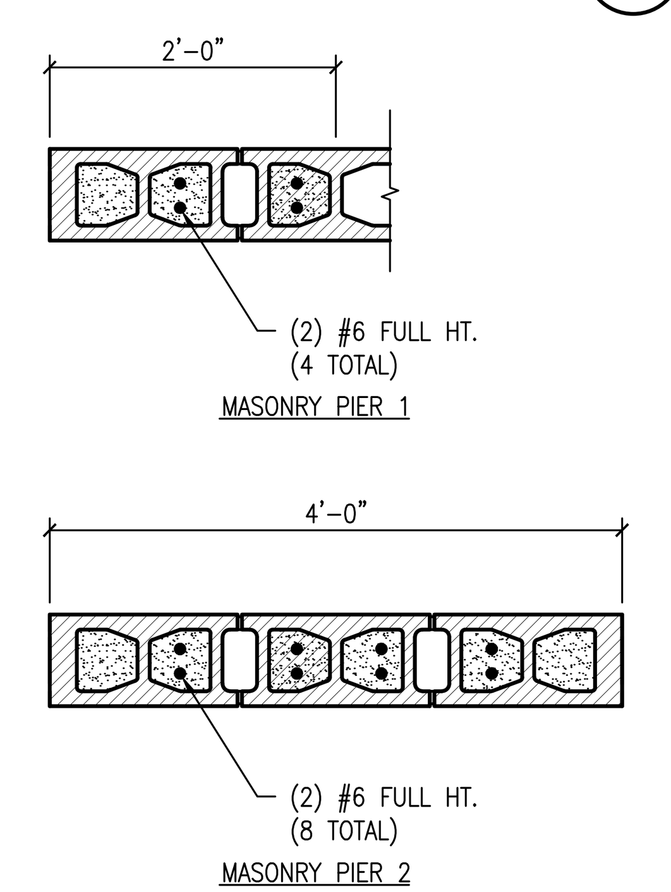
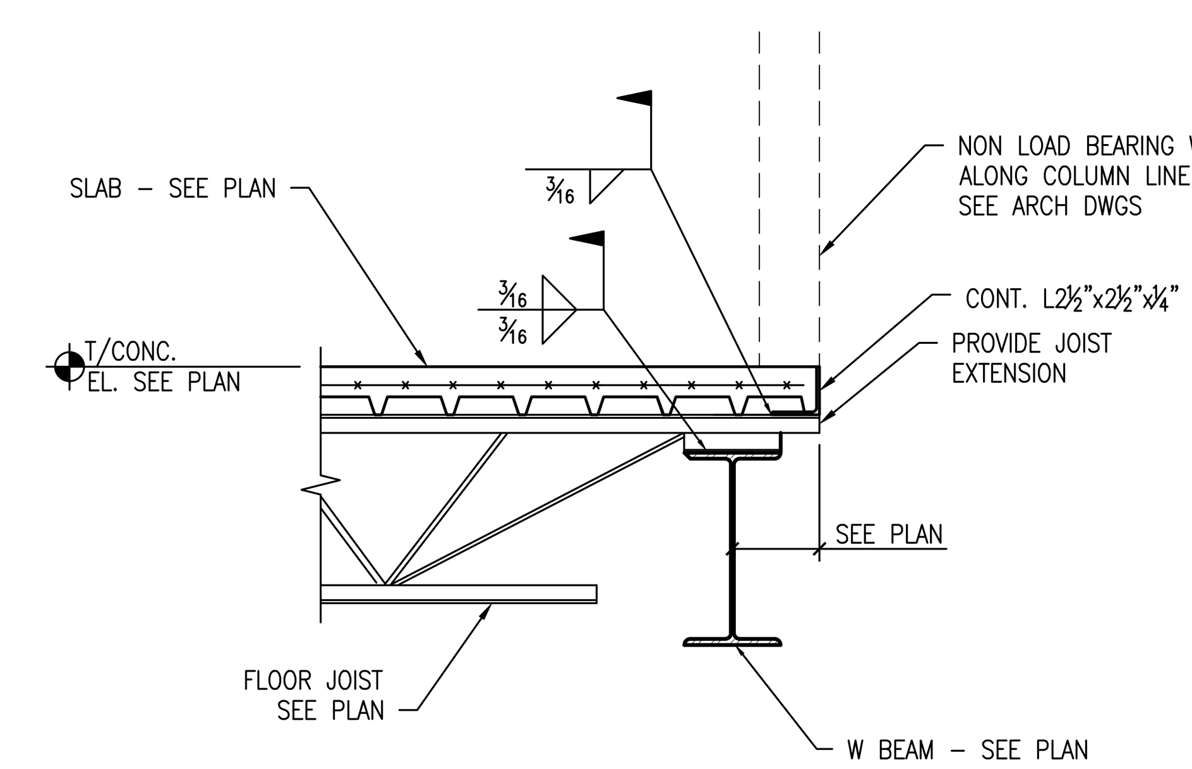


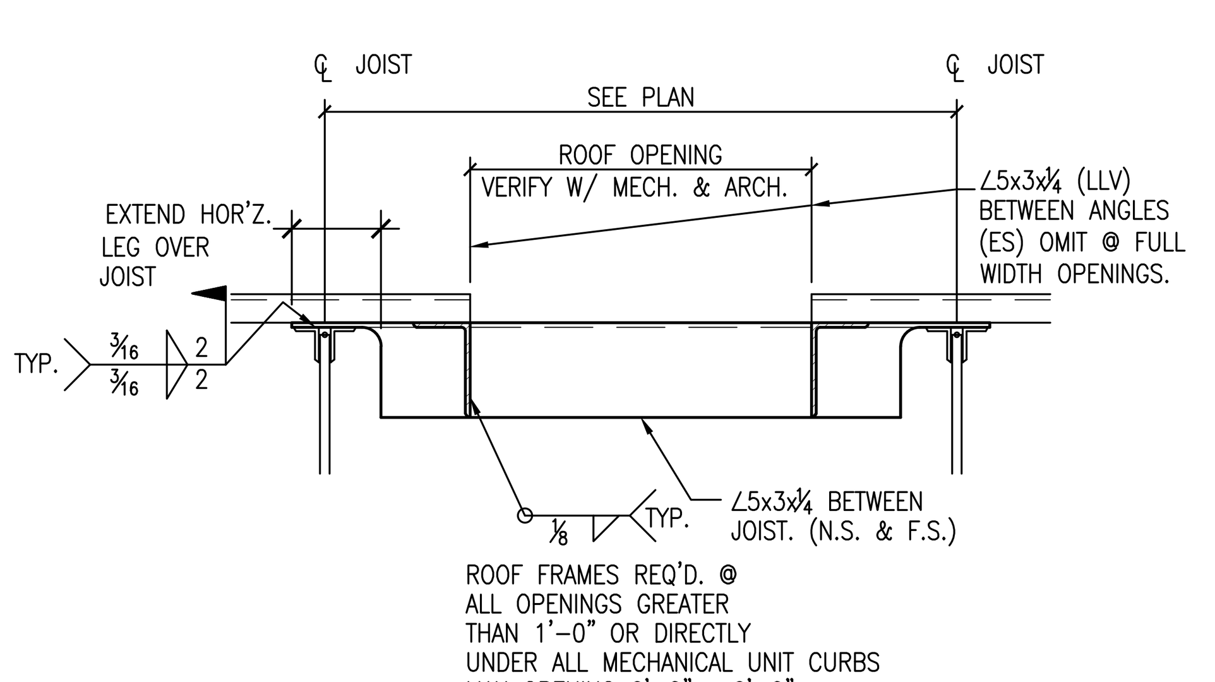
12 SECTION
SCALE: 3/4"=1'-0"



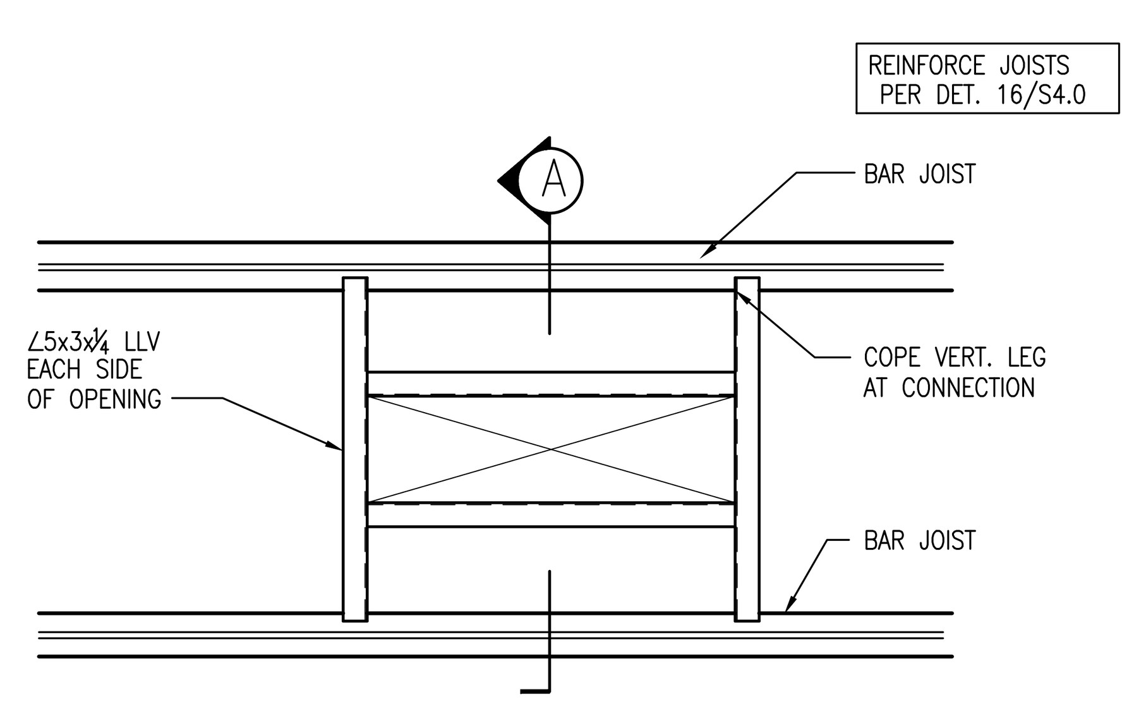
13 MASONRY PIER DETAIL
SCALE: 3/4"=1'-0"



14 SECTION
SCALE: 3/4"=1'-0"

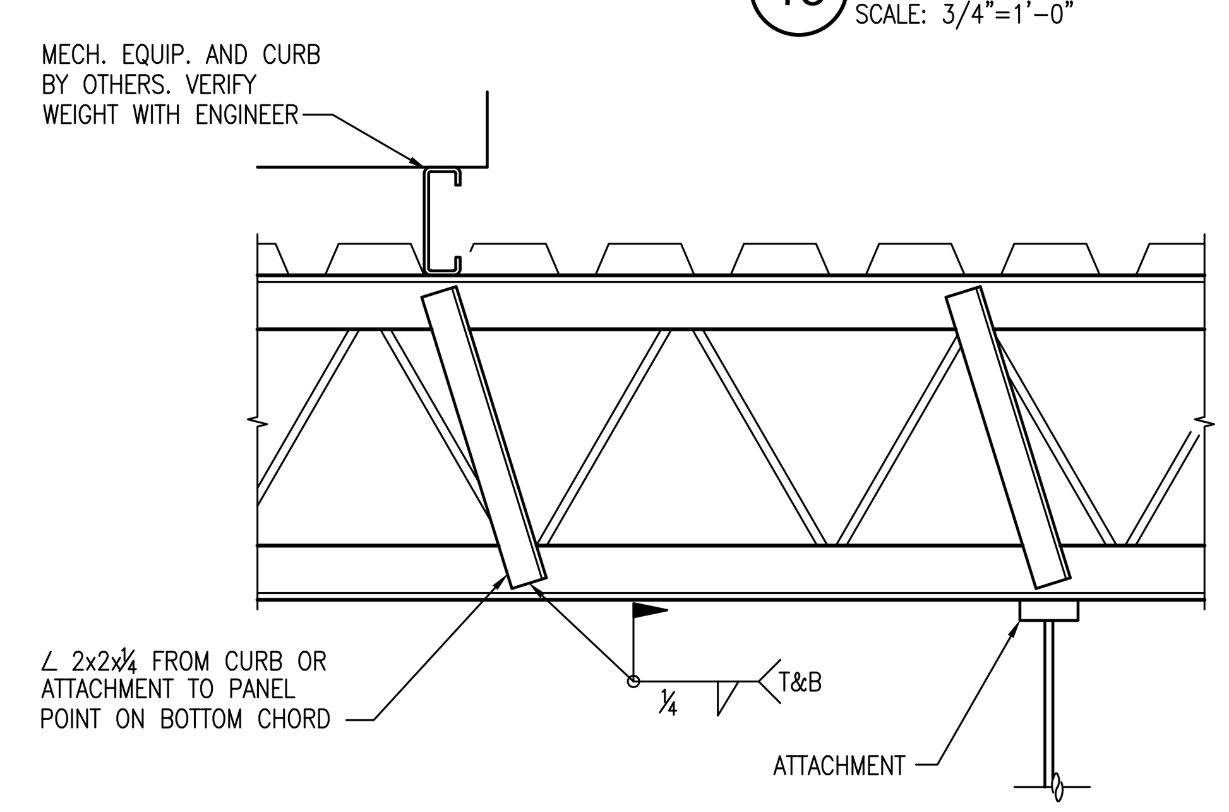


A SECTION



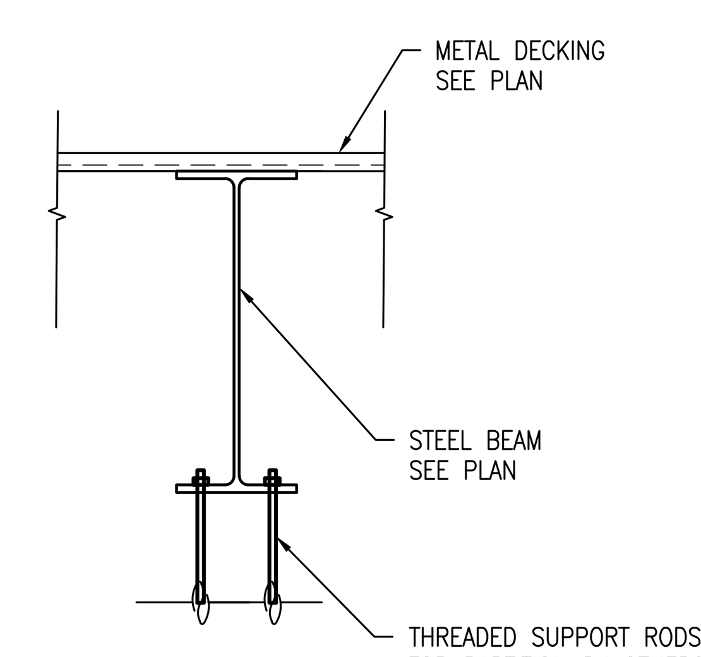
B PLAN

15 TYPICAL ROOF OPENING DETAIL
SCALE: 3/4"=1'-0"

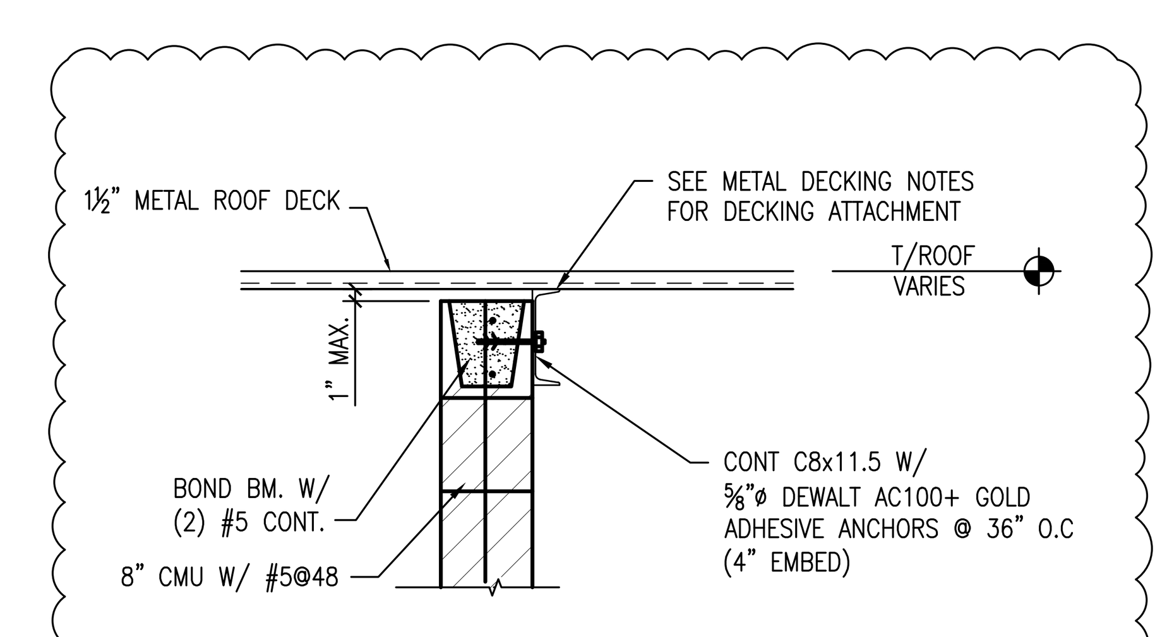


NOTE:
PROVIDE DIAGONAL ANGLE WHERE EVER MECH. EQUIPMENT OR ATTACHMENTS DO NOT LINE UP WITH JOIST PANEL POINT.

16 TYPICAL CONCENTRATED LOAD DETAIL
SCALE: 3/4"=1'-0"



17 PARTITION SUPPORT
SCALE: 3/4"=1'-0"



18 ELEVATOR SECTION
SCALE: 3/4"=1'-0"

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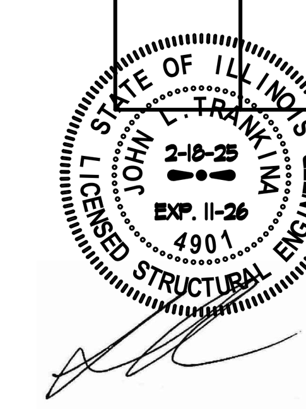

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